GENERAL NOTES 1. Interpretation of Drawings & Specifications a. For convenience, specifications have been prepared for this project and are arranged in several sections, but such separation shall not be considered as the limits of the work required of any separate trade. The terms and conditions of such limitations are wholly between the contractor and his subcontractors, b. In general, the working details will indicate dimensions, position and kind of

construction, and the specifications, qualities and methods. Any work indicated on the working details and not mentioned in the specifications, or vice versa, shall be furnished as though fully set forth in both. Work not particularly detailed, marked or specified, shall be the same as similar parts hat are detailed, marked or specified. If conflicts occur on drawings and/or specifications, the most expensive materials or methods will prevail. C. Should an error appear in the working details or specifications or in work done by others affecting this work, the Contractor shall notify the Architect at once and in writing. If the Contractor proceeds with the work so affected without having given such written notice and without receiving the necessary approval, decision or instructions in writing from the Owner, Then he shall have no valid claim against the Owner, for the cost of so proceeding and shall make good any resulting damage or defect. No verbal approval, decision, or instruction shall be valid or be the basis for any

claim against the Owner, its officers, employees or agents. The foregoing includes typical errors in the specifications or notational errors in the working details where the interpretation is doubtful or where the error is sufficiently apparent as to place a reasonably prudent contractor on notice that, shoùld h'e elect to pr'oceed, he is doing s'o at his own risk.

Construction shall conform to all applicable codes and regulations. Shop Drawing Note: a. When not adressed by division I of the specifications, paper format Structural Shop Drawings shall be submitted in the form of three copies minimum of each sheet

b, The purpose of Shop Drawings submittals by the Contractor is to demonstrate to the Structural Engineer that he understands the design concept by indicating which material he intends to furnish and install, and by detailing the fabrication and installation methods he intends to use on a stand alone set of documents. Duplication of design documents for the ourpose of shop drawings is not acceptable.

Prior to fabrication, Shop Drawings shall be submitted for review by the Structural Engineer. Shop Drawing submittals shall include, but are not necessarily limited to, structural steel, reinforcing steel, & glued laminated d. Prior to submission the Contractor shall review all submittals for conformance with the Contract Documents and shall stamp submittals as being "Reviewed for Conformance". e. Shop Drawings submittals processed by the Structural Engineer are not

Change Orders, f. Any detail on the Shop Drawing that deviates from the Contract Documents shall clearly be marked with the note "This a change". 9. Shop drawings or calculations submitted for review that require resubmittal for re-review shall be billed hourly for such time to the General Contractor. Re-review will not proceed without written approval from the General Contractor for additional engineering review services.

4. Safety Note: a. It is the Contractors responsibility to comply with all OSHA requirements. b. The Contractor shall be responsible for adequate design and construction of all forms and shoring required. Shoring indications (location, direction, duration, etc.) are only shown on the Structural drwgs when required to implement the design intent of the final work product. Determination whether shoring is required for temporary or intermediate conditions during coonstruction is wholly the resposibility of the contractor. c. The Owner and the Structural Engineer do not accept any responsibility for

the Contractor's failure to comply with these requirements,

5. The contractor shall notify the Architect and Structural Engineer where a conflict or discrepancy occurs between the Structural drawings and any other portion of the Contract Documents or existing field conditions. Such notification shall be given to de time so a south dagging the construction shall be a south to a south dagging the construction shall be a south to a south dagging the construction shall be a south to a south dagging the construction shall be a south to a south dagging the construction shall be a south to a south dagging the construction shall be a south dagging the construction shall be a south to a south dagging the construction shall be a south to a south dagging the construction shall be a south to a south dagging the construction shall be a south to a south dagging the construction shall be a south to a south the construction shall be a south to a south the construction shall be a south to a south the construction shall be a south the construction shall be a south to a south the construction shall be a south the construction of the conflict between Structural drawings and specifications the more restrictive condition shall take precedence unless written approval has been given for the least restrictive. Contractor shall verify all dimensions with Architectural prior to

commencing any work. 6. Where no specific detail is shown, the construction shall be identical or similar to that indicated for like cases of construction on this project. Should there be any question, contact the Architect and Structural Engineer prior to

of the existing building shall be kept on the job site. Contractor to obtain these drawings from the Owner (if they are available), 8. Contractor shall provide an allowance equal to 2% of the bid for structural steel, misc, iron and reinforcing steel to be used at the discretion of the Structural

7. When construction attaches to or is within an existing building, a complete set of drawings

9. Any substitutions for structural members, hardware or details shall be reviewed by the Architect and Structural Engineer. Such review will be billed on a time and materials basis to the General Contractor with no guarantee that the substitution will be allowed,

10. Do not scale drawings. Contact the Architect or Structural Engineer for any dimensions not shown. These drawings are not complete until reviewed and accepted by local Building Officials and signed by the Owner and the Structural Engineer.

<u>ABBREVIATIONS</u>

100SN003-1	
AB Anchor Bolt	mfr Manufacturer
abvAbove	MI Malleable Iron
Ы <i>ж</i> Веюж	mtl Metal
bofBottom of Footing	(n) New
brgBearing	NIC Not in Contract
btwnBetween	NS Near Side
ccCenter to Center	nts Not to Scale
CTJControl Joint	NW Normal Weight
CJP Complete Joint Penetration	OH Opposite Hand
cirClear	OSB Oriented Strand Board
CMUConcrete Masonry Unit	pc Piece
CJConstruction Joint	PJP Partial Joint Penetration
contContinuous	PT Pressure Treated
contrContractor	rwd Redwood
cskCountersink	SC Slip Critical
DFDouglas Fir	shtg Sheathing
DLDead Load	sim Similar
doDéda 2000	SMS Sheet Metal Screw
dwgDrawing	SP Structural Panel
(e)Existing	stfnr Stiffener
EFEach Face	stgrd Staggered
EJExpansion Joint	sti Steel
ElElevation	t\$b Top \$ Bottom
ENEdge Nailing	tåg Tongue å G roove
eosEdge of Slab	thrd Threaded
eqEqual	tn Toe Nail
EWEach Way	to Top of
EWEF Each Way Each Face	toc Top of Concrete (slab und
FBFace of Block(or Brick) or	tof Top of Footing or
Flat Bar	Top of Framing
FCFace of Concrete or	tos Top of Steel
Framing Clip(Simpson A35 uno)	tow Top of Wall
FFFinish Floor	uno Unless Noted Otherwise
FSFace of Stud or Far Side	w/ with
FTFire Treated	W/O Without
gaGauge or gage	WP Work Point
glbGlued Laminated Beam	WS Wood Screw
HBHeaded Bolt	WWF Welded Wire Fabric
hdrHeader	£ Centerline
HSBHigh Strength Bolt	£ Plate
HSSHollow Structural Section	WF Wide Flange
htHeight	# Number or Pounds
jhJoist Hanger	sq Square
jhJoist Hanger LLLive Load	ø Round or Diameter

DESIGN CRITERIA

LLV -----Long Leg Vertical

LS -----Lag Screw

LW -----Light Weight

nternational Building Code (IBC) ASCE 7-05 ACI 318-08 AISC 360-05, 341-05, 358-05 TMS 402-08/ACI 530-08/ASCE 5-08 TMS 602-08/ACI 530.1-08/ASCE 6-08 2008 NDS 2. Vertical loads Roof Live Load = 20 psf Live loads are reduced where

Light Weight Insulating Conc

permitted by code. For Administration bldg loads see wood truss notes, 3. Soils Values Allowable soils pressure

) DL + LL <u>2300</u> psf) DL + LL + Seismic <u>3067</u> psf Footing Minimum depth **=** 12° MINIMUM WIATH = <u>16"</u>

Site Class <u>B</u> Cs (Admin) = <u>0.0651</u> Ss = <u>0.122</u>; Sds = <u>0.130</u> S1 = <u>0.055</u>; Sd1 = <u>0.088</u> I = 1.0; Ip = 1.0 Administration Bldg R = 2.0; $\Omega o = 2.5$; Cd = 2.0<u>Bulk Storage Bldg</u> R = <u>2.0</u> ; Ω0 = <u>2.5</u> ; Cd = <u>2.0</u> Occupancy Category: <u>II</u> Seismic Design Category: Seismic Base Shear

Cont Wood in Section

----- "member" above

----- "member" below

✓ ------ Wood Blocking in Section
 ✓ End of Wood Piece

Admin Bldg <u>8.26</u> kips (both dir.) Bulk Storage = 1.88 kips (both dir.) Seismic Force Resisting System; Admin Bldg - light framed walls w/ sureboard Bulk Storage - wood + concrete shear walls Analysis Procedure: Equiv. Lateral Force

Basic Wind Speed = 110 mph Exposure Category CIW- = 1.0; GCpi = ± 0.18 (Administration/Storage)

STATEMENT OF STRUCTURAL SPECIAL INSPECTIONS AND TESTING l. Special inspections and testing shall be provided by an inspection agency,

employed by the owner, and qualified by the building official to inspect the particular type of construction. Tests and inspections, as required by sections 110, 1704, 1707, and 1708 of the 2009 IBC, shall be performed during construction on the types of work listed below: ■ Steel Construction Section 1704,4 & Table 1704,4 Concrete Construction ■ Masonry Construction-Level 1 Section 1704,5 & Table 1704,5,1] Masonry Construction-Level 2 Section 1704,5 & Table 1704,5,3 Section 1704,6 ■ Wood Construction High-Load Diaphragm Section 1704,6,1 Section 1704.7 & Table 1704.7 Driven Deep Foundations Section 1704,8 & Table 1704,8 Cast In-place Deep Foundations Section 1704,9 & Table 1704,9] Driven Deep Foundations] Helical Pile Foundations Section 1704.10

2. Inspections may be continuous or periodic as allowed by the individual material or component inspection sections and tables of section 1704. 3. The special inspector shall submit inspection reports to the building official and the design professional in responsible charge. The reports shall indicate whether work inspected conformed to the Construction Documents. Any discrepancies shall be immediately brought to the attention of the Contractor for correction. If they are professionally and the design profession and the contractor for the attention of the building official and the design professional in responsible charge.

4. All Special Inspection Agencies / Individuals and Shop Fabricators shall be approved by the building official prior to commencement of work. Testing and inspection records shall be retained until completion of construction. 6. The Contractor shall submit a written statement to the building official acknowledging responsibility for construction of the main lateral-force resisting system prior to commencement of that work as required by section 1709 of the 2009 IBC. Special inspections for seismic and wind resistance shall be conducted for all items listed in section 1705.3 and 1705.4 as applicable. Special inspections for seismic resistance shall include all items in section 1707. Special inspections for wind

requirements shall include all items in section 1706. Structural testing for seismic resistance shall be in accordance with section 1708. 8. Masonry Construction Level 2 applies to structures classified as occupancy category IV only. 9. All soils and foundation excavation inspections shall be by the Geotechnical

see construction documents and comply with chapter 17 of the 2009 IBC

1. All foundation work shall be done in accordance with the requirements of the

10. For testing and inspection requirements for non-structural materials and components,

FOUNDATIONS

Soils Report # 870 CM by W.L. Burle Engineers P.A. dated 2/15/11.

2. Foundations shall bear on engineered fill per the soils report

See notes and details on sheet 1000-52

3. All filling, back filling and compaction shall be done under the observation of a representative of the Soils Engineer and must be compacted to the minimum density specified in accordance with the procedure outlined in the soils report. 4. Building pad construction shall conform to the requirements of the soils report. the extent and depth of overexcavation and placement of engineered fill shall at a minimum be as shown on the plans. Final depth and extent of excavation and fill shall be determined at time of construction by a representative of the Soils Engineer, Foundation depths indicated on plans are for estimating purposes only,

5. Bottoms of all foundations shall be level. Changes in bottom of foundation elevation shall be made according to Stepped Footing Detail on the Typical Detail Sheet. Foundation concrete may be placed directly into neat excavations provided the excavations are stable (as 2x12 7 1x8 72x12 determined by a representative of the Soils Engineer). Otherwise, foundations shall be fully

formed. Use minimum planking shown to protect against sloughing, as required. Planking does not replace formwork required to stabilize excavation, . The surface of all horizontal construction joints shall be cleaned & roughened by exposing clean aggregate solidly embedded in mortar matrix. 8. Notify the Structural Engineer 48 hours before casting foundations. 9. A representative of the Soils Engineer shall advise the Building Official in . The building pad was prepared in accordance with the soils report. . The utility frenches have been properly backfilled and compacted and; c. The foundation excavation depth and material are adequate to achieve

design bearing capacity; and forming comply with the soils report and approved plan, CONCRETE . Structural concrete shall attain 28 day compressive strength as required in note #29. Maximum slump shall not exceed 4 inches. 2. Concrete mix designs shall be prepared by a registered Civil Engineer, reviewed by Owner's testing laboratory and submitted to the Structural Engineer for review.
3. Cementitious materials:

Cement shall conform to ASTM C-150 type I or II.
Fly ash shall conform to ASTM C-618, Max, quantity of flyash shall be as given in specs (15% max uno). Concrete aggregates shall conform to ASTM C-33 for normal weight concrete and ASTM C-330 for lightweight concrete, 5. Non-shrink grout or drypack shall consist of a premixed nonmetallic formula. 6. Reinforcina steel shall conform to ASTM A-615 Grade 60 for #4 and larger, and

ASTM A-615 Grade 40 for #3 and smaller, except reinforcing steel to be welded shall conform to ASTM A-706. Contractor shall submit rebar mill certificates.

All preheating and welding of reinforcing bars shall be done in accordance with AMS DI.4 latest edition and shall be continuously inspected by a qualified laboratory. Contractor shall furnish WPS for all rebar welding to the laboratory. 8. Reinforcing steel shall be fabricated according to "Manual of Standard Practice for Reinforced Concrete Construction 9. Wire fabric shall conform to ASTM A-185.

10. Dimensions shown for location of reinforcing are to the face of bars listed and denote clear coverage. Non-prestressed, cast-in-place concrete coverage shall be as follows, uno: Concrete deposited directly against ground (except slabs)--- 3"
Concrete exposed to ground or weather but placed in forms:

#5 and smaller----Beams & Columns (main reinforcing)-----2" Cast-In-Place Walls (exterior face \$ soil side)----- see above Cast-In-Place Walls (interior face-#11 & smaller)----- 3/4" ---- see details Slabs (on forms)------- 3/4"

Slabs (on ground)------2" clear from top uno . Splices in continuous reinforcement shall be lapped uno, see schedule this sheet. Splices in adjacent bars shall be greater than 5'-0" apart. Splice continuous bars in soil-bearing grade beams, structural slabs on grade and mat foundations as follows uno: top bars at centerline of support; bottom bars at mid-span. Splice continuous bars in elevated slabs and beams, etc. as follows uno: top bars at mid-span; bottom bars at centerline of support. All bars size #14 and larger shall be continuous for full length shown or spliced with mechanica couplers as noted in details. Splices in WWF shall be 1-1/2 meshes wide. 12. The minimum clear spacing between parallel bars in a layer shall not be less than the larger of bar diameter, 1", or 33% greater than the maximum aggregate size (nominal), whichever is greatest. This requirement also applies to the clear spacing between different layers of parallel bars and to the clear distance between a contact lap splice and adjacent splices or bars. 13. All hooks shall be standard hooks unless otherwise shown or noted. At walls, provide hooks at ends of all reinforcing at ends, corners and intersections, uno. 14. Construction joints shall be made rough and all laitance removed from the

surface. Concrete may be roughened by chipping the entire surface, sand blasting, or raking the surface to provide 1/4" deep deformations. 15. Remove all debris from forms before casting any concrete. 16. Reinforcing, dowels, bolts, anchors, sleeves, etc. to be embedded in concrete shall be securely positioned before placing concrete. 17. Anchor bolts (AB's) cast in concrete or masonry for wall sill and ledger applications shall be headed bolts with cut threads conforming to ASTM A307, uno. Refer to "Wood" notes for additional requirements for bolts in contac with pressure treated or fire retardant material. Refer to 'Structural Steel noté for requirements for anchor rods (AR's) cast in concrete for column

Malls shall be cast in horizontal layers of 2'-0" maximum depth . Concrete in walls, piers or columns shall set at least 2 hours before placing concrete in beams, spandrels, or slabs supported thereon. Horizontal wall bars in multi-curtain cast in place walls shall be staggerec . Dowel all vertical reinforcing in walls and columns from foundation with same . Consolidate concrete placed in forms by mechanical vibrating equipment supplemented by hand-spading, rodding or tamping. Use equipment and procedures for consolidation of concrete in accordance with the recommended practices of ACI 309 to suit the type of concrete and project conditions. Concrete shall not be dropped through reinforcing steel (as in walls) so as to cause segregation of aggregates. In such cases hoppers and chutes or trunks of variable lengths shall be used so that the free unconfined fall of concrete

23. No wood spreaders allowed. No wood stakes allowed in areas to be concreted. 24. Additional reinforcing in precast or tilt-up panels required for lifting stresses shall be supplied by Contractor.

25. Provide #5 \times 4'-0" diagonal reinforcing at top and bottom of slab at all re-entrant corners typical. This applies to slab on grade, concrete over metal deck, and elevated structural slab conditions. 26. All saw cutting shall be done after initial set has occurred to avoid tearing or damage by the saw blade, but before initial shrinkage has occurred. 27. Notify Structural Engineer a minimum of 48 hours before placing any concrete.

28. CONCRETE STRENGTHS & MIX PROPERTIES: Max Aggr Max W/cm* <u>f'c at 28 days</u> 3000 psi Weight Ratio
NW 0.58 Item A. Foundations 3500 psi B. Slab on grade 0.50 C. Columns, beams, 4000 psi malls, elevated slabs E. Site and miscellaneous 2500 psi

* w/cm = Water : Cementitious material ratio

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base plate and steel embed applications.

shall not exceed 6 feet.

GENERAL NOTES

APPLICABLE TO ALL DRAWINGS UNLESS NOTED OR SHOWN OTHERWISE

							ACI 3 2009	818-08 PIBC		
fc' = 3000 pai conc										
Splice Class	Reinf Location	#3	#4	#5	#6	#7	#8	#9	#10	#11
B	Тор	19	37	47	56	81	93	105	118	131
D	Other	15	29	36	43	63	72	81	91	101
fc' = 3500 psi conc										
Splice Class	Reinf Location	#3	#4	#5	#6	#7	#8	#9	#10	#11
B	Тор	18	35	43	52	75	86	97	109	121
Đ	Other	14	27	33	40	58	66	75	84	93

1. Schedule applies to normal weight concrete with uncoated, Grade 60 reinforcing steel for #4 bars and larger (values for #3 bars based on Grade 40), 2. Top reinforcement is horizontal reinforcement located such that more than

12 inches of fresh concrete is cast in the member below the splice. 3. When lightweight concrete is used, multiply lap lengths by 1.30. 4. Where clear spacing of bars being spliced is less than 2 bar dia. OR where clear cover of bars being spliced is less than I bar dia.,

multiply lap lengths by 1.50, uno. 5. Where notes #3 AND #4 occur, multiply lap lengths by 2.00, uno. 6. Where Class A lap splice is noted in detail, divide lengths above by 1.30.

<u>STRUCTURAL STEEL</u>

l, Fabrication, erection and materials shall conform with the AISC Specification for Structural Steel Buildings, the AISC Seismic Provisions for Structural Steel Buildings, and the International Building Code, latest editions. 2. Structural Steel wide flange shapes shall conform with ASTM A992. All other ructural Steel rolled shapes (channels, angles, etc) and plates shall conform

with ASTM A36, uno. 3. Steel Pipe shall conform to ASTM A53, Types E or S, Grade B. 4. All Hollow Structural Sections (HSS) shall conform to ASTM A500, Grade B. 5. All structural steel shall receive a minimum of one shop coat of red primer paint. Do not paint areas to be field welded, fireproofed, galvanized, to receive alip-critical high atrength bolta, or to be embedded in concrete. Provide

additional painting as noted in the specifications.

10. "Slip-critical" bolted connections:

6. All structural steel shall be erected plumb and true to line. Temporary bracing shall be installed and shall be left in place until other means are provided to adequately brace the structure. Contractor responsible for reviewing all base plate and support conditions during erection and bracing as required. See AISC and OSHA requirements. 7. Place non-shrink grout under all base plates before adding vertical load. 8. Structural steel below grade shall have 3 inches minimum of concrete cover.

9. Bolted connections shall consist of unfinished bolts conforming to ASTM A307 unless noted otherwise. Where high strength bolts are indicated, bolts conforming to ASTM A325 or ASTM A490 as needed shall be provided. Anchor rods cast in concrete or masonry shall be headed bolts with cut thread, full diameter body style conforming to ASTM F1554 gr. 36, 55 (weldable per SI Supplementary Requirements), or 105 as indicated on drawings. All bolted connections and base plates shall have standard cut washers unless noted otherwise. Washers for base plates shall conform to ASTM F844 unless noted otherwise, and shall be placed at top and bottom of plate.

a) "Slip-critical" connections (A325SC design values with special inspection) aré required at all braced frame connections, at all connections along chord lines and drag lines (as noted on plans), and uno, at all bolts in oversized or slotted holes, b) The special inspector must be present during installation and tightening operation of "slip-critical" connections. 11. Provide 1/2" diameter stitch bolts and ring fills, spaced at not more than 2'-0" on center for all double angle members. 12. At wood to steel parallel contact, bolt with 1/2" diameter bolts at maximum

13. Holes for unfinished bolts shall be of the same nominal diameter of the bolt plus 1/16". Use standard AISC gage and pitch for bolts except as noted 14. Welding shall be done by the electric arc process in accordance with American Welding Society Standards, using only certified welders. All groove welds shall have complete penetration unless noted otherwise. All exposed welds shall be ground smooth. All electrodes for welding shall comply with AWS code, E70

15. Weld lengths called for on plans are the net effective lengths required, 16. Minimum fillet welds: 3/16" @ t < 1/2" 1/4" @ t < 3/4"

5/16" @ t > 3/4" 17. Welding Procedure Specifications (WPS) for shop and field prequalified weld joints and weld joints qualified by test shall be prepared for review prior to fabrication. All welding procedure items such as base metals, welding processes, filler metals and joint details that meet the requirements of AWS DI. Section 5.1 shall be considered as prequalified. Any change or substitution that is beyond the range or tolerance or requirements for prequalification shall be qualified by test per AWS DI.I Section 5 part B. Qualification testing is required when the depth of a partial penetration or complete penetration weld is 2" or greater.

18. For nondestructive testing of welded connections excluding primary members of moment resisting frames: a) Welded connections shall be tested by nondestructive methods for compliance with AISC J2, and job specifications. Ultrasonic Testing shall be in accordance with AWS D1.1, ASTM E164 and ASME Section V. Radiography shall be in accordance with AWS DI.1, ASTM E94 and E99, and ASME Section V. This testing shall be part of the special inspection requirements of IBC Section 1704.3 performed by an approved independent testing laboratory as follows:

. Base metal thicker than I inch when subject to through thickness weld shrinkage strains, . All complete joint penetration groove or butt welds. . All partial joint penetration groove welds when used in column splices. b) Any material discontinuities shall be accepted or rejected on the basis of defect rating in accordance with the (larger reflector) criteria of AISC J2.

GLUED LAMINATED BEAMS

1. Typical Glued Laminated beams shall be manufactured from visually graded Western species and shall conform to the following combinations: Simple Span Members: 24F-V4
Cantilever and Continuous Members: 24F-V8
Exterior Members not protected from weather shall be Alaskan Yellow Cedar (AYC) or as indicated above with an approved preservative treatment. Where AYC members occurs, they shall conform to the following cominations: Simple Span Members: <u>20F-V12</u> Cantilever and Continous Members: <u>20F-V13</u>,

2. Camber all beams on as indicated on plans. Where cantilever members are cambered, the member shall only be cambered at the interior span. No camber at cantilevers, typical unless noted otherwise.

 Each Glued Laminated beam shall be inspected by a qualified inspector and stamped with a quality control mark signifying it was manufactured according to ANSI/AITC AI9O.I. 4. All Glued Laminated beams shall be fabricated using wet-use adhesives complying with ANSI/AITC A190.1.

<u>COLD FORMED METAL FRAMING</u>

Galvanized sheet steel shall conform to ASTM A653, structural quality, with a minimum yield strength of 33 ksi for 43 mils (18 ga) and thinner and ASTM A653, structural quality, with a minimum yield strength of 50 ksi for 54 mils (16 ga) and thicker. Hot-rolled carbon sheet and strip steel used in the fabrication of cold-formed members shall conform to ASTM AIOII with a rust inhibitive coating. 2. Metal studs and joists shall be of size and thickness shown on drawings with the minimum effective section properties shown in the table(s). 3. Minimum thickness shown in table for the thickness specified represents 95% of design thickness per 2011 SSMA w/ 2004 supplement

4. Metal framing shall be per ICC-ES No. 4943P. Contractor shall be responsible for obtaining agency approval for any substitutions.

5. Welding shall be in accordance with AWS DI.3 "Structural Welding Code-Sheet Steel". Welders shall be AWS certified. Welding Rods: E60XX series. All field welding shall have special inspection,

6. Typical metal track shall be same gauge as studs which it supports, unpunched, with a flange width of 11/4 inches and a depth equal to the nominal stud plus 2 times the track thickness plus the radius. Nested tracks shall be fabricated to fill the outside of a typical metal track. Deep leg tracks shall have a minimum flange width of 2 inches. Use slotted slip tracks where specified. See sections and typical

1. Metal studs shall not have punch-outs closer than 10" from the end of the stud or at intermediate lateral bearing points of studs.

1. All structural wood shall conform with the following specifications: Douglas Fir - Coast Region - WCLIB grading rules #17 or WWPA standard aradina rules f*or western* lumber 2x, 3x and 4x members - #2 uno. 6x and larger members - #1 uno. 2x decking - Select Dex Members 3x and larger shall be free of heart center Redwood - California Redwood, RIS. Sheathina - US Product Standard PS 1-95 and PS2-92.APA rated Struct 1/sheathing w/ exterior glue @ walls, floors, & roofs uno Pressure Treated Douglas Fir - type as appropriate for exterior above ground use as specified, by AMPA. All wood in direct contact with concrete shall be pressure treated, except ledgers, which do not need to be pressure treated.

3. Field cuts and bolt holes in pressure treated wood shall be protected in accordance with AWPA standard M4. 4. Bearing and shear walls shall have double top plates, lapped at wall and partition intersection with 3 - 16d nails. Splice upper and lower plates as in "Typical Stud Wall & Opening Framing Detail" on Typical Detail Sheet 1000-57. 5. Provide solid blocking between joists and rafters at all supports. Provide blocking at all ceiling levels. 7. Joists under and parallel to partitions shall be doubled and nailed together. 8. The moisture content of 2x material at time of initial use shall be less than

19%. The moisture content of lumber 3x and larger at time of initial use shall be less than 30% uno. 9. Holes for bolts in wood shall be bored with a bit of the same nominal diameter as the bolt plus 1/16" 10. Holes for lag screws shall be first bored to the same diameter and depth as the unthreaded shank. The threaded portion shall be drilled to 50% of

the shank diameter unless noted otherwise in the specifications. II. Lag screws and wood screws shall be screwed and not driven into place. Soap may be used to lubricate screws. 12. All bolts and lag screws shall be provided with metal washers under heads and nuts which bear on wood. Applies also to expanding and adhesive fasteners. Square steel washers shall be Simpson BP or BPS type (3" sq. min. w/ std cut washer where req'd by code). Malleable Iron (MI) washers shall be round and cast. Standard flat washers shall be per ANSI B18.22.1 type A - wide pattern. All washers for sill anchors shall besquare steel and shall be hot-dipped galvanized. 13. All bolts and lag screws shall be tightened on installation and retightened before

closing in or at completion of job. 14. All bolts shall be full nominal dimension @ unthreaded portion. No upset threaded 15. All fasteners except lag screws & bolts shall be hot-dipped galvanized or stainless steel where in contact w/ pressure treated or fire retardant material.

16. Use of machine nailing is subject to satisfactory review for each project and subject to approval by the governing agency. The approval is subject to continued satisfactory performance, Machine nailing will not be approved in 5/16" sheathing. If nail heads penetrate the outer ply by more than would be normal for a hammer or if minimum allowable edge distances are not maintained, the performance will be deemed unsatisfactory.
17. Block shtg joints with 2x4 flat blocking where noted on roof or floor framing plans and with blocking same size as stude at walls. Use plyclips at midepan of unsupported roof sheathing edges.

18. Framing hardware shown on the plans is Simpson Strong-Tie, Use framing hardware as manufactured by Simpson Company or equivalent. Prior to installation of any non-Simpson hardware, the contractor shall submit a list of all detail references where a hardware substitution is proposed, the designation for the Simpson item and non-Simpson proposed equivalent and an ICC report for each substitution item.

19. Lay all structural sheathing on roof and floors with long dimension perpendicular to supports unless noted otherwise. 20. Notify Structural Engineer after wall, floor, and roof shtg nailing has been completed and a minimum of 48 hours prior to concealing nailing.

All nails for structural work shall be common wire nails uno, conforming to the following minimum sizes:

- 0.131" diameter x 2-1/2" -0.148" diameter x 3" 10d sheathing - 0.148" diameter x 1-5/8" plus thickness of shtg - 0.148" diameter x 1-1/2" -0.162" diameter x 3-1/2" - 0.192" diameter x 4" -0.148" diameter x 4" 20d box

Holes shall be sub-drilled where necessary to prevent splitting. Nailing not noted below or on plans shall be a minimum of two nails at each contact. 8d for Ix material and 16d for 2x material.

-0.162" diameter x 5"

I. Joists or Rafters	- Sawn lumber brg on plates I-joists brg on plates Lapped to sides of studs	TN each side
2. Studs to Bearings	-2x4 studs	TN each side 2-10d
	2x6 and 2x8 studs	EN in lieu of TN 2-16d TN each side 3-10d EN in lieu of TN 3-16d
	2x10 and 2x12 studs	TN each side 4-10d EN in lieu of TN 4-16d
3. Blocking	· between joists, rafters or studs between joist or	TN each side each end 2-10d EN in place of TN 2-16d TN ea side 3-10d
4. Ledgers	rafter beårings (plates) - to studs	ledger 2-8d ledger 2-16d
5, Double Top Plates	- lower plate to 2x4 stud lower plate to 2x6 or 2x8 s	2-16d tud 3-16d See "Typical Stud Wall & Opening Framing" Detail
6. Sill Plates	to parallel framing	16d @ 12"cc each contact 2-16d
7, Multiple Studs 8, Built Up Beams	stagger for widths more the 2-2x members, top and bott	an 4" 16d @ 12"cc om 16d @ 12"cc clinched 1/2" diameter bolts at 12"cc staggered
9, Ceiling Stripping	holes shall be pre-drilled for nails at ends of stripping boards Where ceiling is plaster or ! Ring Nails (no slant),	1x 2-8d (1-slant) 2x 2-16d (1-slant)
10. Shear Wall Nailina	See	"Shear Wall Nailina" detail

10, Shear Wall Nailing -------- See "Shear Wall Nalling" aetail 12. Non-Structural -----at edges and interior HDG casing nails ------6d @ 6"cc supports 13. Non-Structural ----- at edges Nails to match shear walls @6"cc Nails to match shear walls @ 12"cc Sheathing at field

14. Use deformed shank nails for all floor sheathing, 15. All exposed fasteners shall have a zinc-coating corrosion resistive surface. 16. Nails used into pressure treated or fire retardant material shall be hot-dipped galvanized per ASTM A123 or stainless steel.

<u>SIMPSON HARDWARE</u>

NAILING SCHEDULE

1. Simpson hardware for this project has been specified using 100% of the allowable load values and product information given in the Simpson catalog for the year that these drawings are dated. Plans, sections and details indicate general product type and any required modifications (i.e.; slopes and skews). Specific catalog call outs are determined

by individual member sizes. Examples:
Plans/sections indicate 2x10 members supported by Simpson 'U' hangers.
Simpson call-out: U210. Plans/sections indicate 5-1/8"x18" glb supported by 4x6 post and Simpson ECC cap. Simpson callout: ECC5 \$1/4-4 or option with straps rotated 90° ECC5\$1/4-6

3. Where the Simpson catalog indicates two sizes may be used for the same size member, use the larger size. Example:
Plans/sections indicate 4x8 member supported by Simpson LUS hangers. Simpson catalog indicates LUS46 or LUS48 may be used. The LUS48 shall be usėd uniess noted otherwise.

4. Use Simpson 'max' nailing configuration ("fill all holes with nails"), typical unless noted otherwise. 5. Where multiple nail sizes are indicated in Simpson catalog, use largest nail size listed, typical unless noted otherwise. 6. Hardware supplier other than Simpson shall submit a comparative material list itemizing product designation, load rating, and supported member size for review by the enforcement agency and the Structural Engineer.

DRILLED-IN ANCHORS

. Epoxy anchors shall be Hilti RE500-5D per ESR-2322 or Simpson SET-XP per ESR-2508 for thr'd rod & rebar. Expansion anchors shall be Hilti KB-TZ per ESR-1917 or Simpson Strong-Bolt per ESR-1711. Type, size & embedment shall be indicated in drawings. Post-installed anchors for repair shall be evaluated on a case by case basis. Notify Structural Engineer for repairs.

2. Anchors shall be installed in accordance with the requirements given in the 3. Unless noted otherwise anchors have been designed for special inspection. Provide Special Inspection as indicated in the ICC report. 4. When installing drilled-in anchors in existing concrete or masonry, use care and

caution to avoid cutting or damaging existing reinforcing bars. Do <u>not</u> install anchors in prestressed concrete elements) 5. The inspection of the anchors shall be done by a qualified inspection agency and a report of the inspection results shall be submitted to the governing agency and Architect/Structural Engineer.

Drawing Title

GENERAL NOTES

WOOD ROOF TRUSS

Roof Design Loads

___10___ psf LL (nonconcurrent w/top chord LL)

Net Wind Uplift Load for Zone 1 - 14 psf

Net Wind Uplift Load for Zones 2, 3 Net Wind Uplift Load for overhangs - 112 psf

Wind load zones defined in ASCE 7-05 fig 6-11B through 6-17. Discontinuity distance 'a' = 3'-10'' ft.

All members shall be $2x4 \min (uno)$. All top chord members shall be of material w/a minimum specific gravity of 0.50. Plate material shall be ASTM A-446 Grade A or better.

 Design and fabrication shall conform to the code as designated in the design criteria notes and ANSI/TPI I-2007. 5. Design and construction of connections shall be in accordance with published approvals of manufacturer's ICC-ESR. Heel joint design shall consider the effects of eccentric loading in accordance with Truss Plate Institute (TPI).

6. Truss designer shall oversize plates for chord members to account for wood defects like knots, knot holes and greatly distorted grains. Maximum allowable defect size per member shall be 2 square inches. No defects allowed under plates for web members. 'Truss layout & profile drawings, calculations and the latest ICC approved test data for truss metal plate connectors shall be submitted to the Architect

and/or Engineer for review and shall address all comments from that review prior to submittal to the bldg dept. 8. Truss layout, profile drawings and calculations for truss designs and any bearing detailing per note 20 shall be signed by a Professional Engineer registered in the State in which the project is located. It shall be the respect to 10 the manufacturer to obtain Building Department approval

prior to fabrication. 9. General Contractor to determine and provide temporary erection bracing as required. 10. Built up girder trusses shall be laminated using 1/2"@ bolts at 24"cc

maximum through all members (or other method by written request). II. Gable end trusses shall have 2x verticals at 16"cc typical unless noted otherwise. Where gable end trusses are required to span over openings verticals shall be infilled around typical web members. 12. All hardware required for connecting trusses (Jack to Hip, Hip to Girder or Girder to Girder, etc.) shall be designed, detailed and provided by truss designer/

13. Truss manufacturer to verify all dimensions shown on Structural drawings with Architectural drawings and in field with wall layout prior to fabrication. Provide shop drawings which shall include plan drawing showing truss locations and truss profiles, with dimensions reviewed and approved by General Contractor orior to fabrication.

14. Truss designer to account for the weight and lateral restraint forces of all equipment in the design of all trusses which support such equipment. See other discipline's drawings for weights and detailing. Additionally, truss designer shall incorporate weight and seismic restraint forces of all piping and conduit in truss design. See other disciplines drwgs for type and location 15. Truss designer shall incorporate one 400# (150#DL, 250#LL non-concurrent)

vertical load on the bottom chord of all trusses from one sprinkler line. russ must be designed to carry the load at any point along the top and bottomchords, 16. Truss designer/manufacturer shall clearly designate truss member bracing

locations in the shop drawings. Fire sprinkler installer shall use connectors with ICC approval.

18. Truss manufacturer shall provide marking on each truss as indicated per ANSI/TPI 1 2007, 19. Structural design of beams, posts & foundations supporting roof trusses has been based on framing configurations shown on plans. Deviations to the framing configurations shown will require additional design coordination and potential revisions to framing and foundations. Any required revisions to the structural drawings will be preformed on a time and material basis and will be charged to he aeneral contractor.

20. The structural drawings generally indicate the wind uplift connection detailing @ the truss supports. Truss manufaturer shall review the structural drawings for uplift design and shall make a statement on the shop drawing submittal stating brought to the attention of the structural engineer for review and input during the shop drawing submittal process.

21. Truss manufacturer shall provide designs that do not exceed a bearing stress of 600 psi 0 the reaction points. At truss manufacturer's option, bearing plates or other connectors as required to reduce bearing stress 0 supporting member to 600 psi formax may be designed, detailed, and submitted for review to the Architect/Structural Engineer.

TYPICAL PREFABRICATED METAL BUILDING NOTES

l. Design and fabrication shall conform to the 2009 International Building Code (IBC), and the latest editions of AISC "Specifications for the Design, Fabrication, and Erection of Structural Steel Buildings", and AISC "Specification for the Design of Cold-Formed Steel Structural Members".

Metal Building Manufacturer (MBM) shall be AISC category 'MB' certified.

Drawings, calculations and engineering data on structural sections for all components shall be submitted to the Architect and/or Structural Engineer for review prior to fabrication. See specifications section 13-34-19 for loading information. 4. Calculations shall be signed by a Civil Engineer registered in the state in which the project is located. 5. Building manufacturer shall provide plan drawing showing column locations and

anchor bolt locations prior to fabrication. Anchor bolt sizes, numbers, and locations are to be designed and detailed by MBM. MBM shall furnish required anchor bolts and setting templates. 6. Contractor shall verify all dimensions with Architectural drawings and MBM column layout prior to foundation construction.

10. Foundation design is based on preliminary evaluation of metal building reactions.

 All hardware required for connecting building components shall be designed, detailed and provided by building manufacturer. B. Contractor shall provide temporary erection bracing as required. 9. Building designer shall account for the weight of all mechanical equipment in the design of all building components which support such units.

Final building reactions are to be submitted to the Structural Engineer for

METAL BUILDING FOUNDATION NOTES

validation of foundations prior to construction.

ı, Metal Building Manufacturer (MBM) selected by owner shall provide calculations for approval. MBM to provide column reactions and anchor bolt layout to verify

2. Foundation design is based on assumed wind pressure at the project site along with design criteria shown on this sheet.

3. Anchor bolt size and location determined by MBM. Embedment and anchorage to concrete has been determined by Foundation Engineer. 4. Contractor shall verify all dimensions with the architectural drawings and MBM anchor bolt layout prior to foundation construction.

POWDER ACTUATED FASTENERS (SHOT PINS)

shot pins.

1. These notes govern all conditions called out on the plans as 'shot pins' unless specifically noted otherwise. 2. All shot pins shall be X-U Universal Knurled Shank Fasteners with shank diameter of 0.157" as manufactured by Hilti Incorporated in accordance with ICC ESR-2269 and the current edition of the Hilti 2008 'Product Technical Guide' 3. All shot pins shall include P8 steel washers.

4. Shot pin's driven into steel base material shall maintain a minimum edge distance at all steel elements of 1/2" and minimum fastener spacing shall be 1". Length of pin shall be as required to penetrate thru steel member u.n.o. At 3/4" thick steel, penetration need not exceed 1/2". 5. Shot pins driven into concrete base material shall maintain a minimum edge distance at all concrete elements of 3" and minimum fastener spacing shall be 4". Pins shall have 11/4" penetration u.n.o. Minimum concrete thickness shall be 3 times the penetration depth. Concrete shall attain full design strength prior to installing

6. Shot pins driven into 3 1/4" minimum light weight concrete fill over 3"x 20 ga minimum metal deck may be installed from the top or from the bottom in either the high or low flute. Pins installed from the top shall be spaced as noted above for typical concrete elements. Pins installed from the bottom in the high flutes shall be installed within I" of flute center. Pins installed from the bottom in the low flutes shall be installed within I" of the flute center and shall be no closer than 1 1/8" to the edge of the low flute. Pins installed from the bottom shall be spaced no closer than 5.1/2" parallel to the flutes. Pins shall have 1" penetration into concrete

u.n.o. Concrete shall attain full design strength prior to installing shot pins.

7. Shot pins may be driven into 8" nominal minimum thickness fully grouted normal-weight cmu with type 5 mortar and minimum f'm = 1500 psi at time of installation. Shot pins may be installed into the face shells, horizontal mortar joints or vertically centered in the top of grouted cells. Shot pins shall not be installed in vertical mortar joints or within I'' of Vertical mortar joints. No more than one shot pin may occur in an individual masonry unit cell and must be installed a minimum of 4'' from the edge of the wall. Shot pins in mortar joints must be a minimum of 8''

from the end of the wall and shall have a minimum spacing of 8".

8. Shot pin installers shall be certified by Hilti and have a current Hilti issued operators license. Shot pin installation shall meet all OSHA requirements.

CONSTRUCTION DOCUMENTS

1000-S1

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MARCH 22, 2012 LOUISIANA NATIONAL CEMETERY **GRAVESITE EXPANSION PHASE 1B**

ZACHARY, LOUISIANA

870CM3023B Approved: Director, Office of Construction Management Checked DRAWING NO. BOR GAM Approved: Director, Project Management Service LOUISIANA NATIONAL CEMETERY

BRIAN REIL No. 4337 Exp. 3/31/13 STRUETURAL



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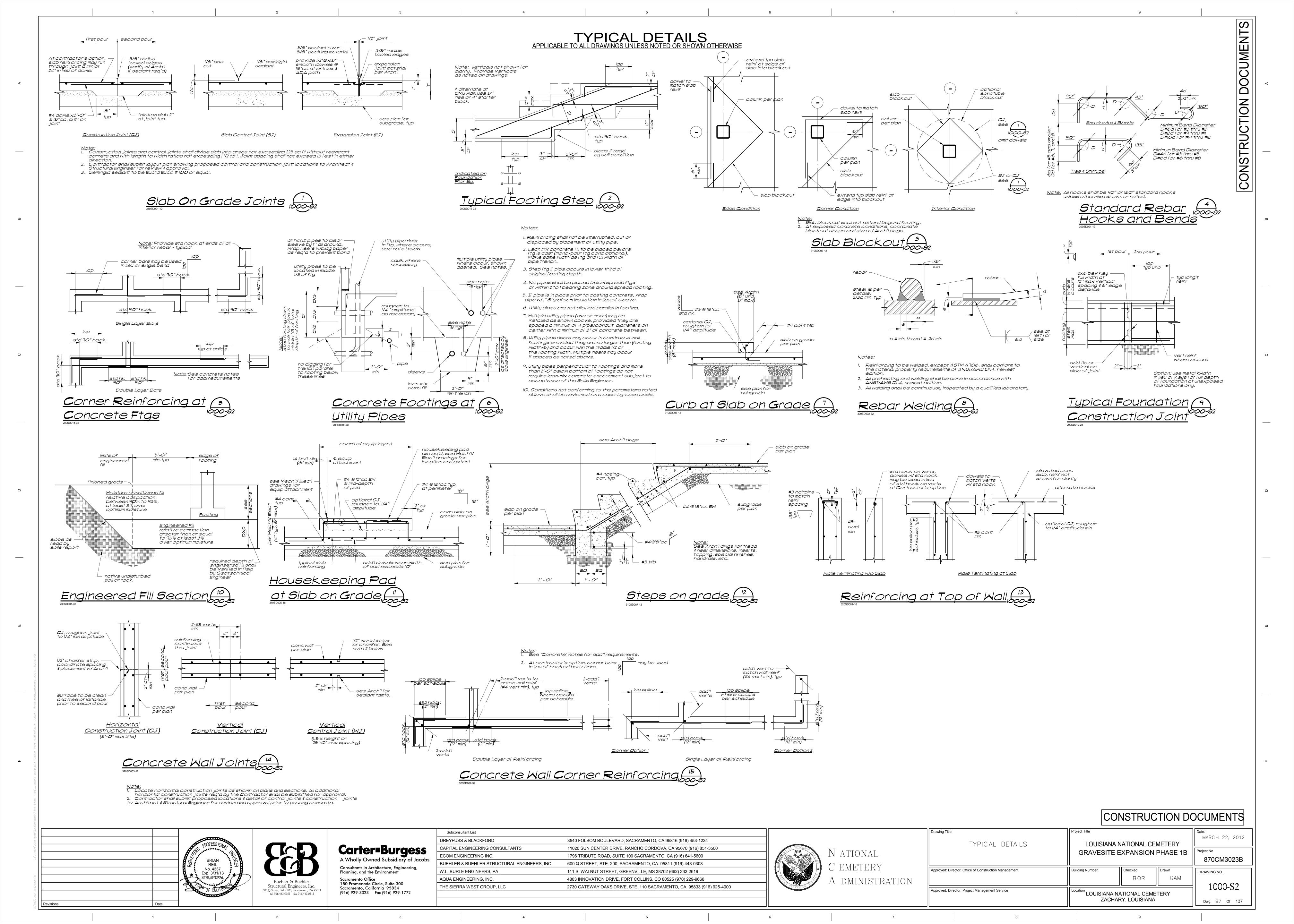
Sacramento, California 95834

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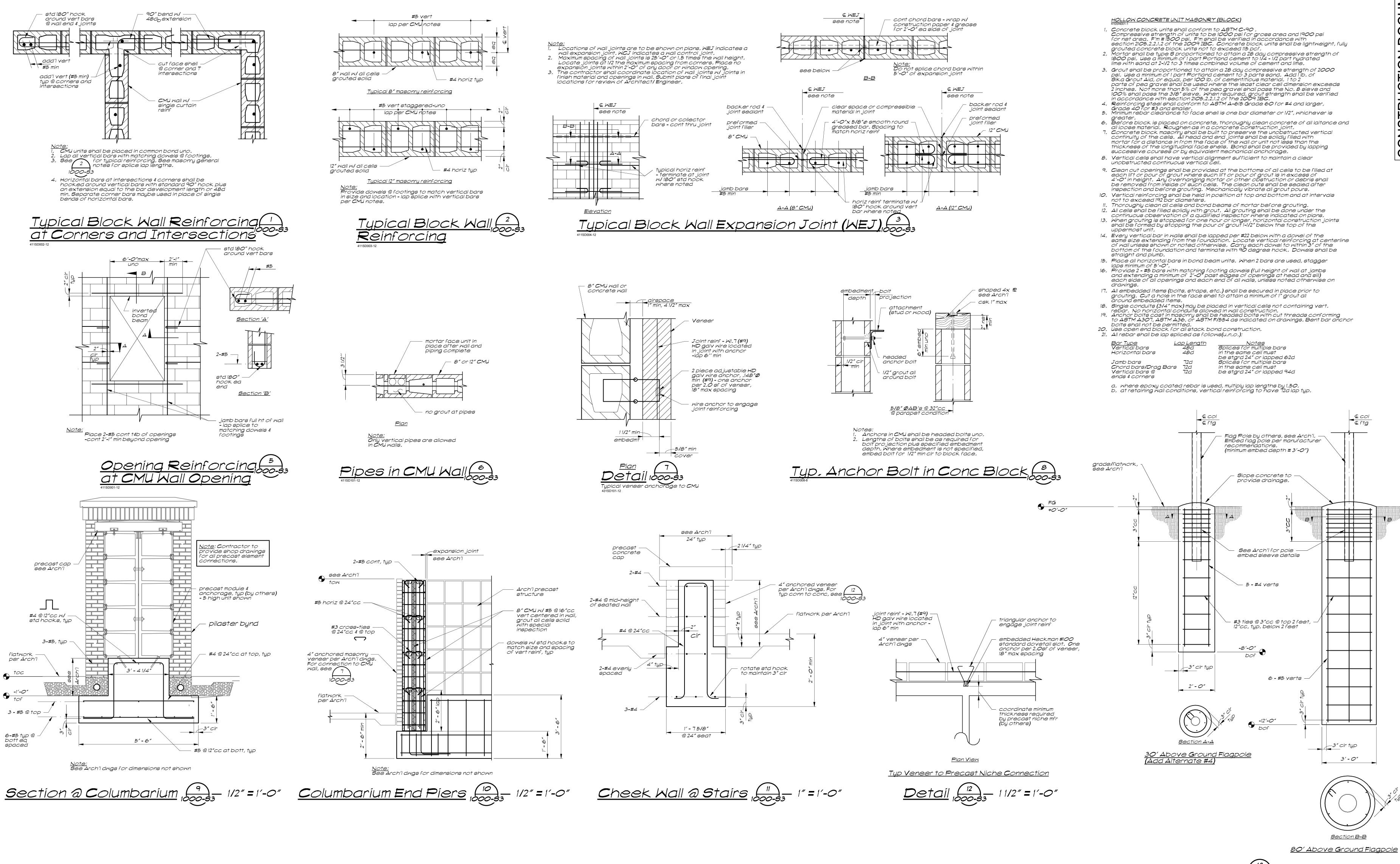
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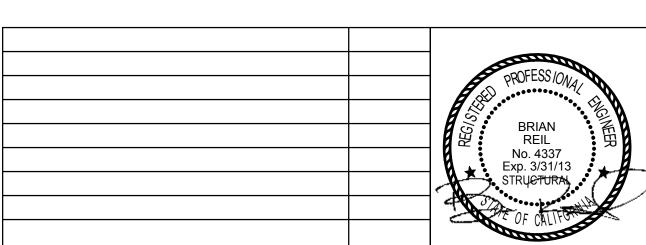


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TYPICAL DETAILS APPLICABLE TO ALL DRAWINGS UNLESS NOTED OR SHOWN OTHERWISE







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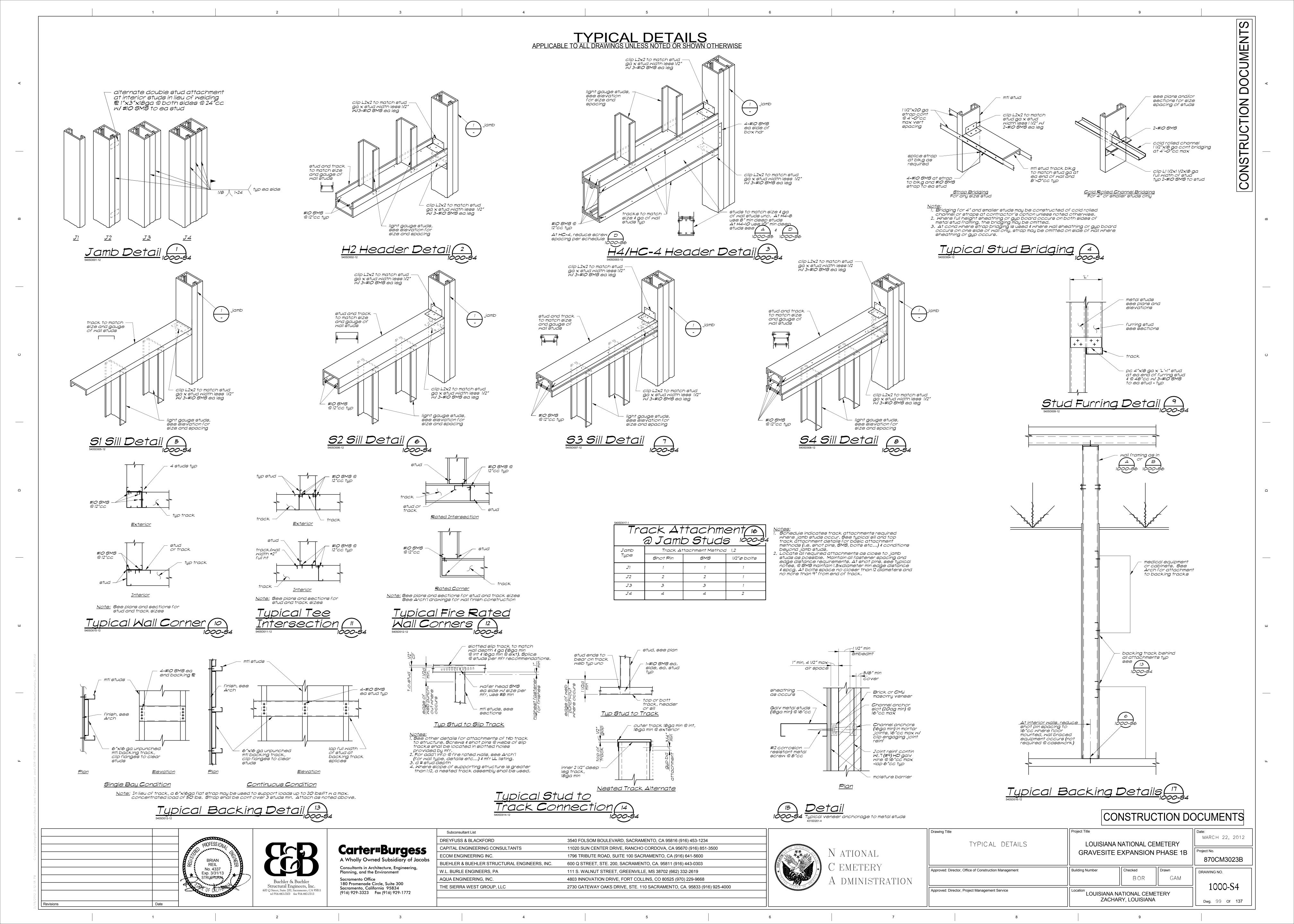


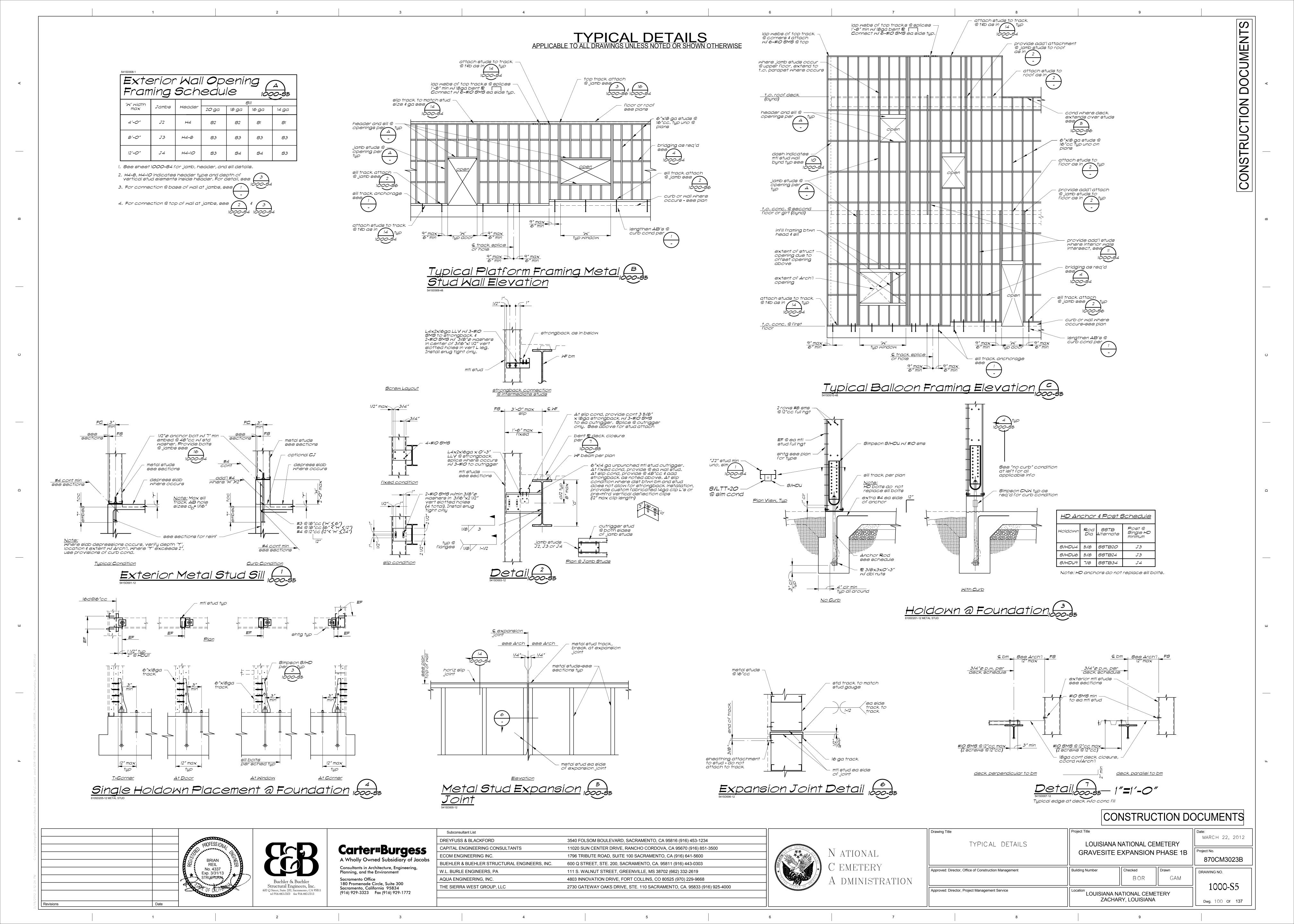
Approved: Director, Project Management Service

Drawing Title TYPICAL DETAILS	Project Title LOUISIANA N GRAVESITE E	NATIONAL CEI	
Approved: Director, Office of Construction Management	Building Number	Checked B O R	Drawn (

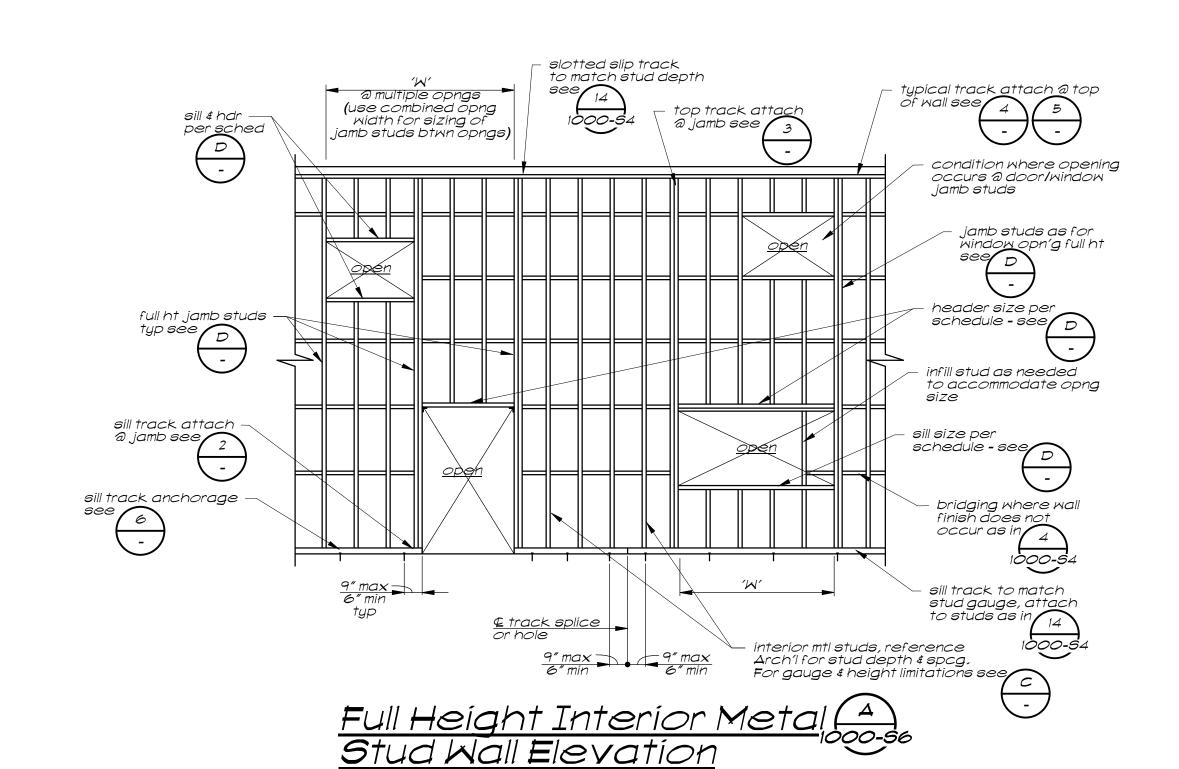
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CONSTRUCTION DOCUMENTS





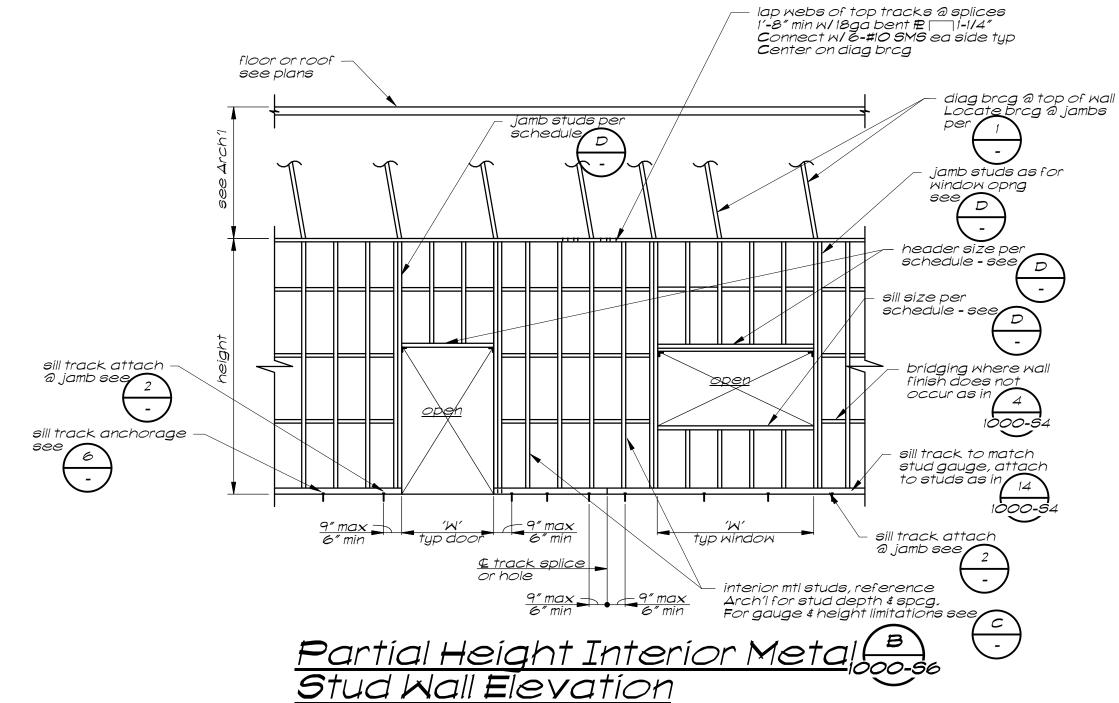




Perpendicular Condition

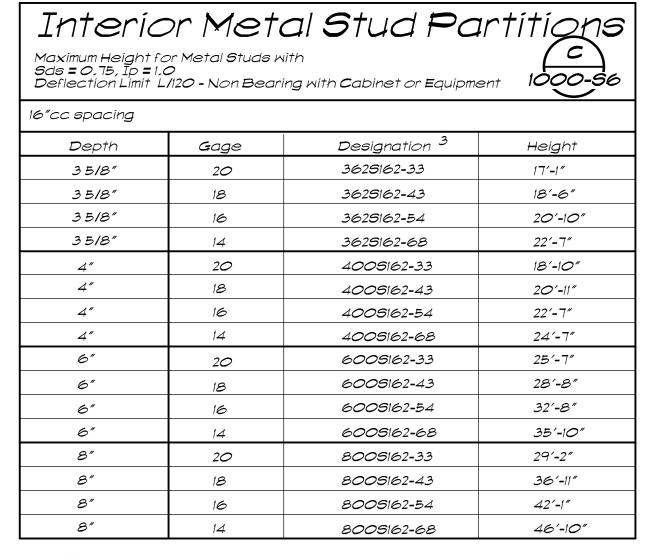
metal deck

#10 SMS in low flute of



TYPICAL DETAILS

APPLICABLE TO ALL DRAWINGS UNLESS NOTED OR SHOWN OTHERWISE



 Stude shall be depth as indicated on Arch drawings and gauge as determined by height of wall and the schedule above. 2. See elevations \triangle \clubsuit for typical wall framing

3. Designation conforms to Steel Stud Manufacturers Association standards.

4. For wall supported cabinet installation see \int_{17}

conditions.

5. The max, hts noted in this table are based upon the use of seismic component load Fp or a live load of 5psf, whichever governs.

'W' width max $_{12}$ Jambs Header Sill HC4-8 Screw Spcg $3-5/8'' \stackrel{?}{\epsilon}$ $6'' \stackrel{?}{\epsilon} 8''$ $4'-0''$ J1 H2 SI $8'-0''$ J2 H4 SI $12'-0''$ J3 HC4-8 9 82 8"CC 12"CC $16'-0''$ J4 HC4-8 94 84 8"CC 12"CC	Fran	rior P ning S Limit = L/180	Nall C Sched	peni: dule	ng (D 00-56
4'-0" J1 H2 S1 Walls Walls 8'-0" J2 H4 S1 S 12'-0" J3 HC4-8 S2 8"cc 12"cc		Jambs	Header	Sill		·
8'-0" J2 H4 SI 6 12'-0" J3 HC4-8	12					
12'-0" J3 HC4-8 52 8"CC 12"CC	4'-0"	J1 ¹	H 2	5 1		
12'-O" J3 HC4-8 52 8"cc 12"cc	8' - 0"	J2	H4	51 ⁶		
	12'-0"	J3	2,9 HC4-8	5 2	8″cc	12″cc
	16'-0"	J4		54 ⁷	8"CC	12″cc

1. Use J2 minimum at door openings

2. H4-8 may be used at 12'-O" opngs in 16ga and 14ga walls w/ the exception of 8" walls

3. Use 18ga min. headers at 16'-0" opngs in 3-5/8" \$ 4" walls 4. Use 16ga min, headers at 16'-0" opngs in 6" walls

5. Use 14ga headers at 16'-0" opngs in 8" walls

6. Use 52 sills at 8'-0" opngs in 3-5/8x20ga walls 7. 52 may be used at 16'-0" opngs in 6" \$ 8" walls (18ga min)

8. See sheet 51.3 for jamb, header, and sill details

9. "H4-8" indicates header type and depth of vertical stud elements inside header. "HC4-8" indicates

composite header w/ add'l screw requirements - see schedule \$ 1000-54 10. For connection @ base of wall at jambs see -

where ceiling joists span — more than 12'-O", provide

clg joist -

schedule

Typical Ledger Condition

- ceiling joist per schedule

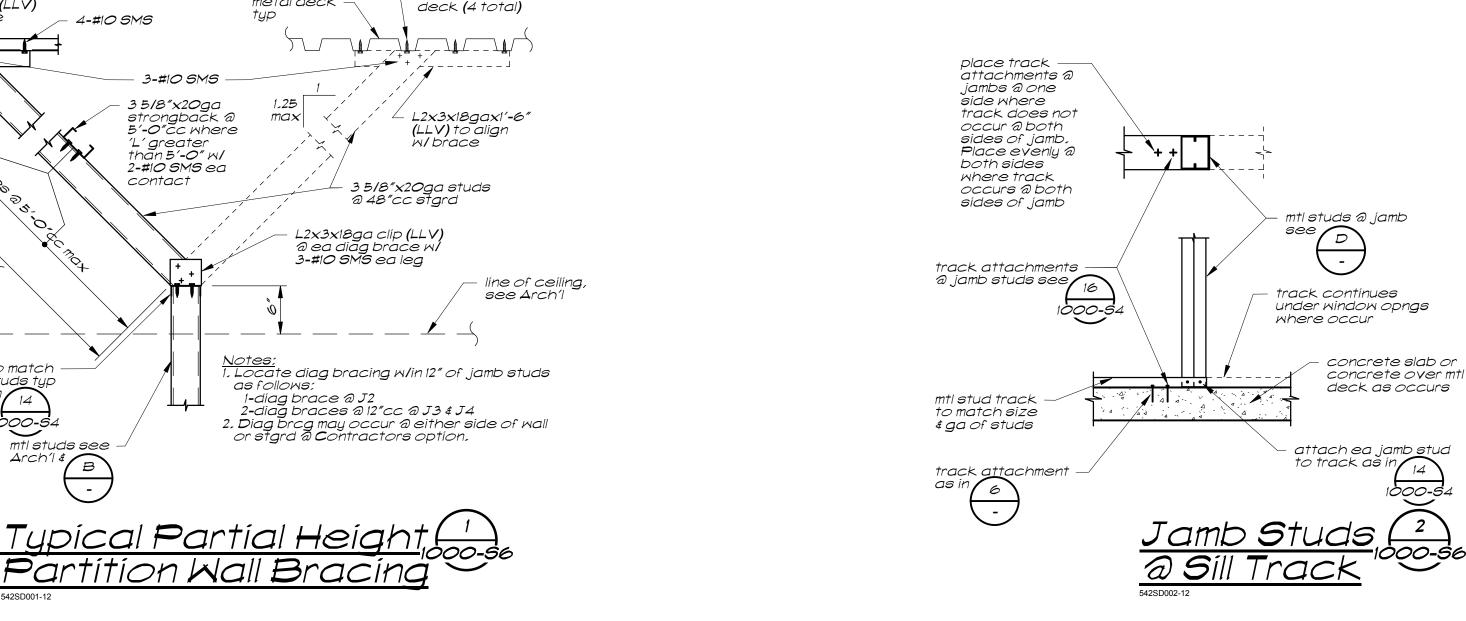
clip L2x2x18ga x joist depth less 1/2" w/ 3-#10

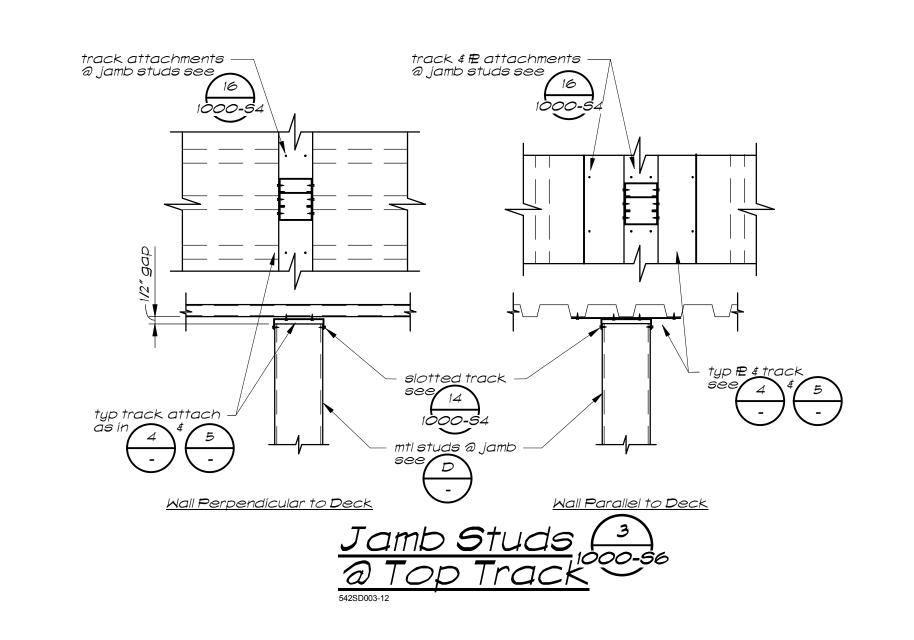
SMS ea leg

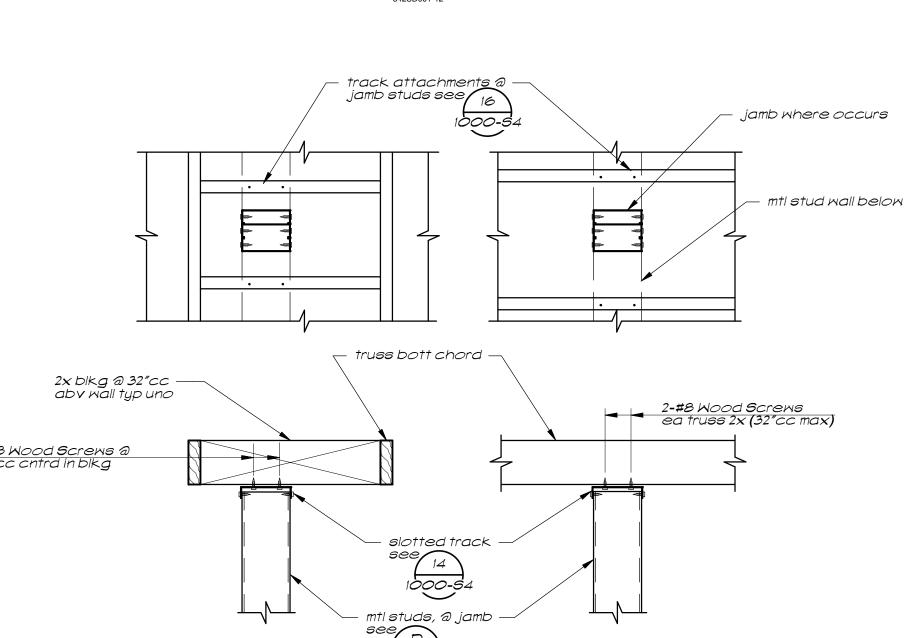
bridging at third — points, 4'-0"cc max as in 4 typ

11. For connection @ top of wall at jambs see

12. 'W' width is the max width of a single opening or the combined width of side by side openings that share a single jamb stud configuration.







1000-50

Parallel Condition

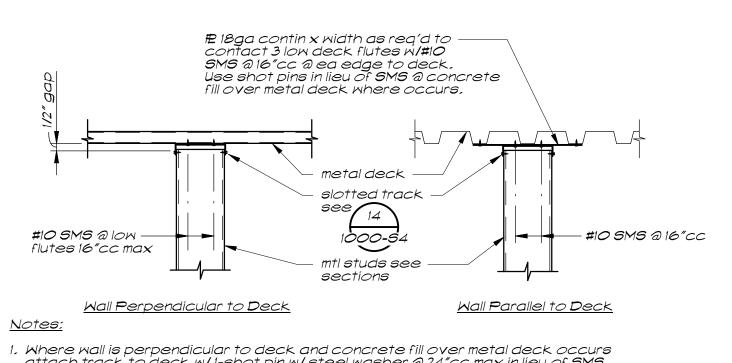
std track to match -

mtl studs see -Arch'l & B

gauge of stude typ attach as in 14

- L2x3x18gax1'**-**6" (LLV)

to align w/brace

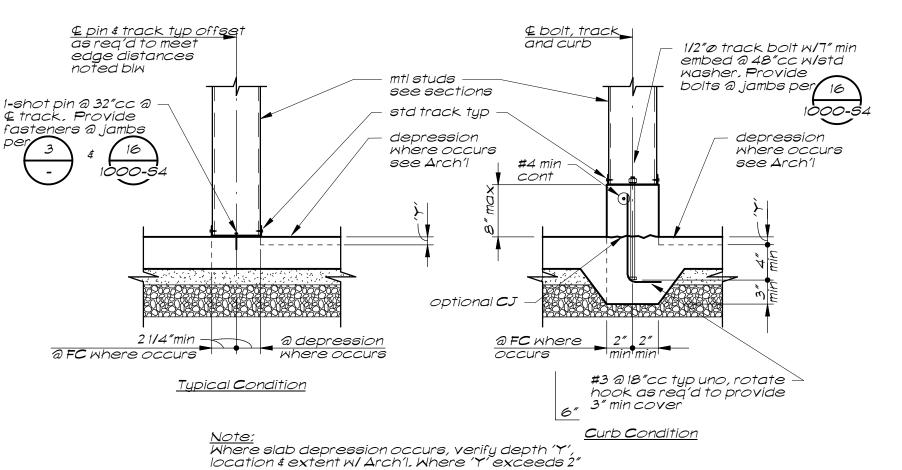




3. See shot pin notes for spacing, edge distance \$ placement requirements. At SMS maintain 1.5 \times diameter min spacing \$ edge distance.

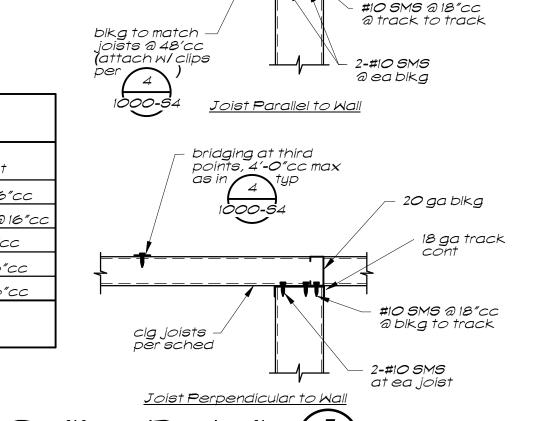
Deck w/o Fireproofing

4. For cond @ jamb studs see $\sqrt{3}$ Slip Track to Metallogo-



use provisions of curb cond. Sill Track to Slab

C	Ceiling Joist Sched	dule
Non according	Accoccibio	
Non-accessible Span	Accessible Span	ceiling
10'-0" max	5'-0" max	21/2" × 18 g
14'-0" max	8'-0" max	35/8"×1
	10'-0" max	6" x 20 gc
19' -0" max		
19'-0" max 20'-0" max	12' -0" max	6" x 18 g



18 ga track cont м/ 3-#10 5М5 @ ea

stud. Where track

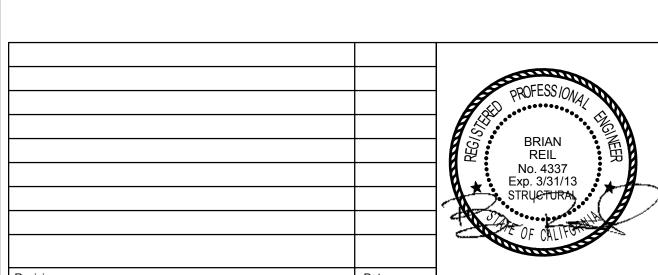
18 ga track cont

attaches to struct steel, use shot pins in lieu of SMS

Typical Framed Ceiling Details

Jamb Studs Wall 600-56 Conn @ Top Track

<u> Wall Parallel to Trussess</u>





Wall Perpendicular to Trussess

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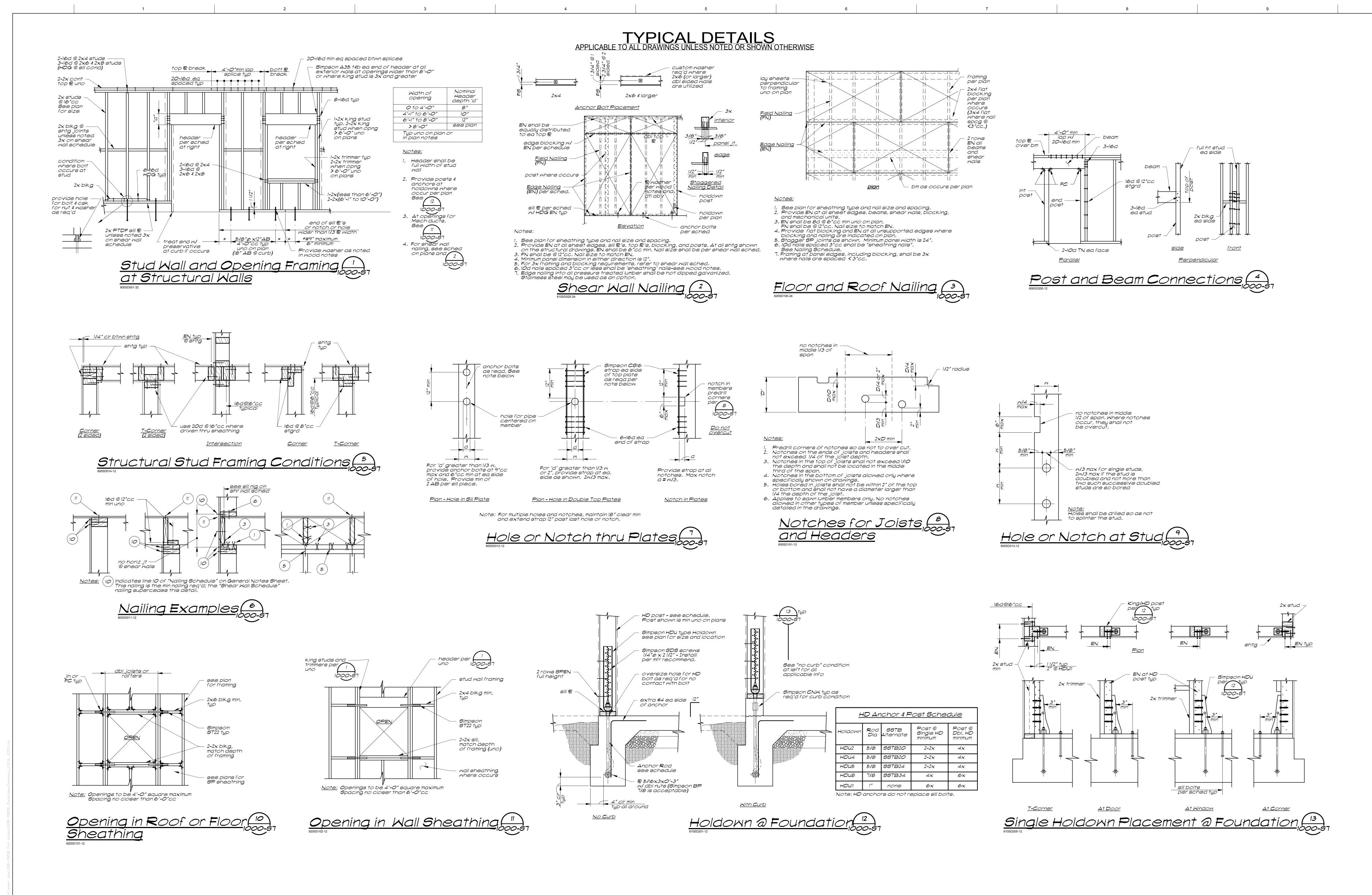
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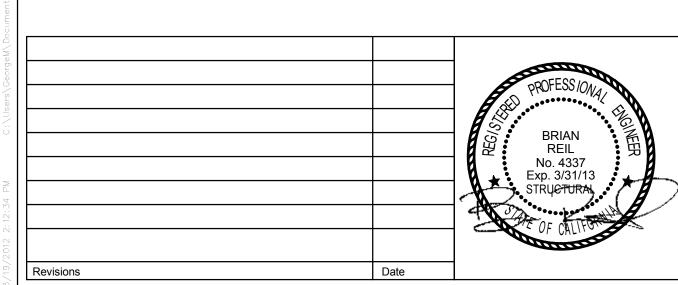


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Drawing Title TYPICAL DETAILS	Project Title LOUISIANA N	NATIONAL CE	METERY	Date: MARCH 22, 2012
	GRAVESITE E	Project No. 870CM3023B		
Approved: Director, Office of Construction Management	Building Number	Checked B O R	Drawn GAM	DRAWING NO.
Approved: Director, Project Management Service	LOUISIANA NA ZACHAF	Dwg. 101 Of 137		



CONSTRUCTION DOCUMENTS

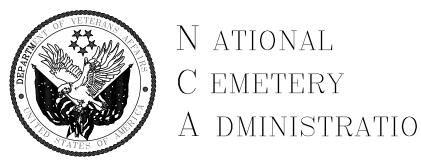
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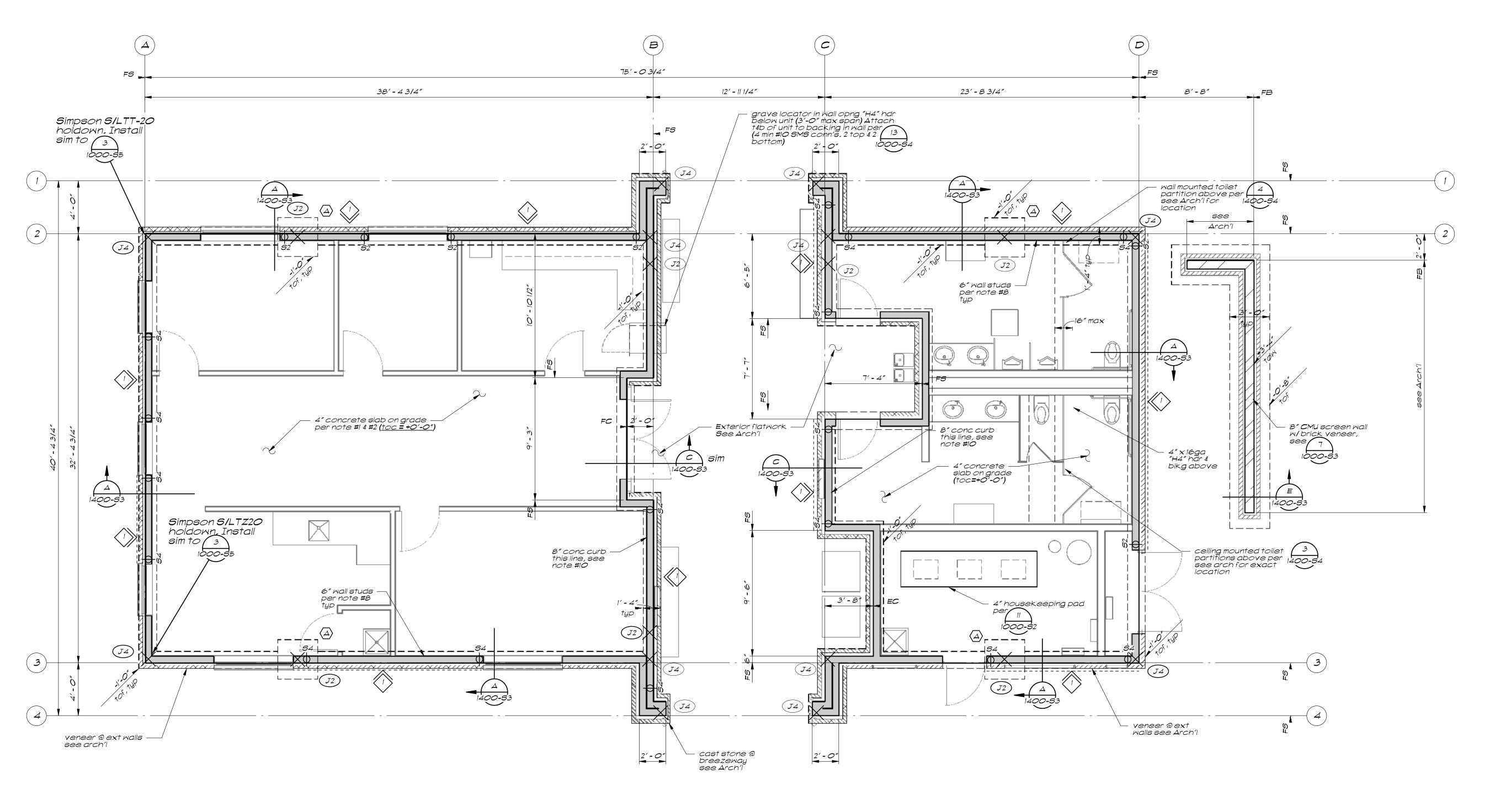
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,	BUEHLER & BUEHLER
Consultants in Architecture, Engineering, Planning, and the Environment	W.L. BURLE ENGINEER
Sacramento Office	AQUA ENGINEERING, I
180 Promenade Circle, Suite 300 Sacramento, California 95834	THE SIERRA WEST GR
(916) 929-3323 Fax (916) 929-1772	

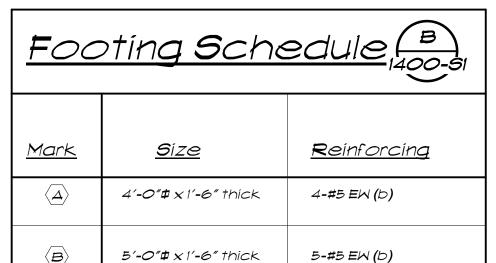
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•	BUEHLER & BUEHLER STRUCTURAL ENGINEERS, INC.	600 Q STREET, STE. 200, SACRAMENTO, CA. 95811 (916) 443-0303	DEP.
	W.L. BURLE ENGINEERS, PA	111 S. WALNUT STREET, GREENVILLE, MS 38702 (662) 332-2619	
	AQUA ENGINEERING, INC.	4803 INNOVATION DRIVE, FORT COLLINS, CO 80525 (970) 229-9668	
	THE SIERRA WEST GROUP, LLC	2730 GATEWAY OAKS DRIVE, STE. 110 SACRAMENTO, CA. 95833 (916) 925-4000	
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TYPICAL DETAILS	LOUISIANA N GRAVESITE E	Date: MARCH 22, Project No. 870CM302		
rector, Office of Construction Management	Building Number	Checked B O R	Drawn GAM	DRAWING NO.
rector, Project Management Service	LOUISIANA NA	1000-9		

ZACHARY, LOUISIANA





<u>Notes:</u>

1. Rebar shall be 3" clear from bottom and sides of footing, 2"

Metal Stud Shear Wall Schedule (A)

(MK)	Sheating Material	Sheathing Edge Fasteners (EF)	Mtl Stud Track \$ blkg shear trans Fasteners	Anchor Bolting	Allow Shear F
	1/2" SP 1 side	#8 x 1 1/4" long 'bhs' @ 3"cc	#12×2" SMS @ 6"CC	1/2″ © a 32″cc	1158 plf

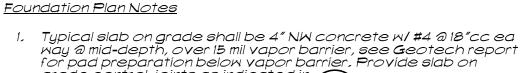
- A. Sure-Board Series 200 Structural Panel. Install per mfr. recommendations and ER-0126. See ER for gyp board backing reqm'ts and water resistant reqmt's.
- B. Metal stud top and bottom tracks shall be 16ga with 1 1/4" minimum flanges. Metal studs shall be 18ga with 1 5/8" minimum flanges.
- C. For shear wall fastening and framing requirements see $\binom{2}{100}$ All sheathing fasteners shall be 1" long minimum. 1400-54
- E. Use Δ -307 M.B. (no 'J' bolts) with standard cut washers between bolt head and track see

D. See sections and details elsewhere in drawings for location.

- 1000-55
- F. Allowable shear based on léga minimum framing.
 G. Provide mtl stud/track blkg @ all shtg joints per 4.1000-54
- H. Sure-board panels shall only be installed at shear walls. All other walls to receive typ gyp board per Arch'i.



P.I.C./Administration Foundation Plan - 1/4" = 1'-0"



grade control joints as indicated in /

typical for all interior slabs. Construction joints may replace control joints as required. Reference top of concrete (toc) shall be +0'-0" typ uno. See Civil plans for datum elevation. Verify all dimensions with Architectural drawings. Notify Architect/Engineer of any discrepancies for resolution prior to proceeding. Dimensions are to face of stud (FS), uno. Face of concrete lines up w/face of shtg.

Indicates shear wall type, see Indicates pad footing per

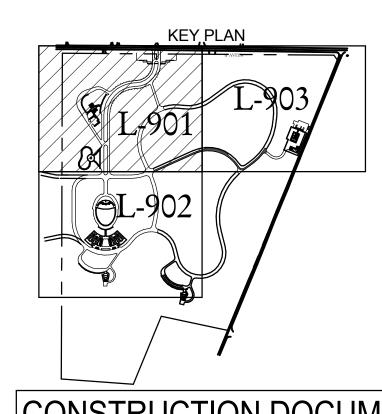
Indicates HSS steel column. See plan for size. HSS4x4x1/4 typ uno. Indicates léga built-up metal stud - see $\begin{pmatrix} 1 \\ 1 \end{pmatrix}$ for type. See plan for type callout and loc'n, 1000-54

Indicates 6"x 18ga metal studs per plan, typ uno, All shear walls to have Sure-Board Series 200 Structural Panels, See sheets 1000-54 \$\pm\$ 1000-55 for typical details, See \(\frac{1}{4} \) for fastener requirements,

- 9. Indicates 8" CMU wall. For reinf & typical details see sheet 1000-53
- Indicates 6" high curb below wall uno, See () \$
 for typ contruction and Architectural 1000-52 drawings for curb width. 11. iggraph Indicates Simpson S/HDS type holdowns, see igspace
- (i.e. 58 **=** HD8'5). Indicates elevation of top of footing below reference finished floor (toc) +0'-0", see plan.
- 13. Provide 3" minimum concrete cover at base £'s, AB's, and all other exposed steel below grade, typ.
- 14. For utility line (pipe & conduit) penetrations through footings see 6
- 15. See Architectural and Civil drawings for grades, finishes and other requirements of all exterior flatwork, curbs, planters,

16. See Architectural drawings for door and window details and locations, blockouts at door closers, curbs, depressions,

pedestal dimensions, slopes and other like items. 17. Refer to sheets 1000-51 to 1000-57 for General Notes \$ Typical Details which are applicable to all drawings, uno.



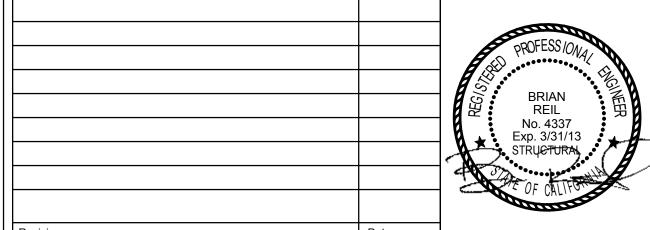
CONSTRUCTION DOCUMENTS

Checked

LOUISIANA NATIONAL CEMETERY

ZACHARY, LOUISIANA

BOR





Carter Burgess
A Wholly Owned Subsidiary of Jacobs Consultants in Architecture, Engineering, Planning, and the Environment Sacramento Office 180 Promenade Circle, Suite 300 Sacramento, California 95834

(916) 929-3323 Fax (916) 929-1772

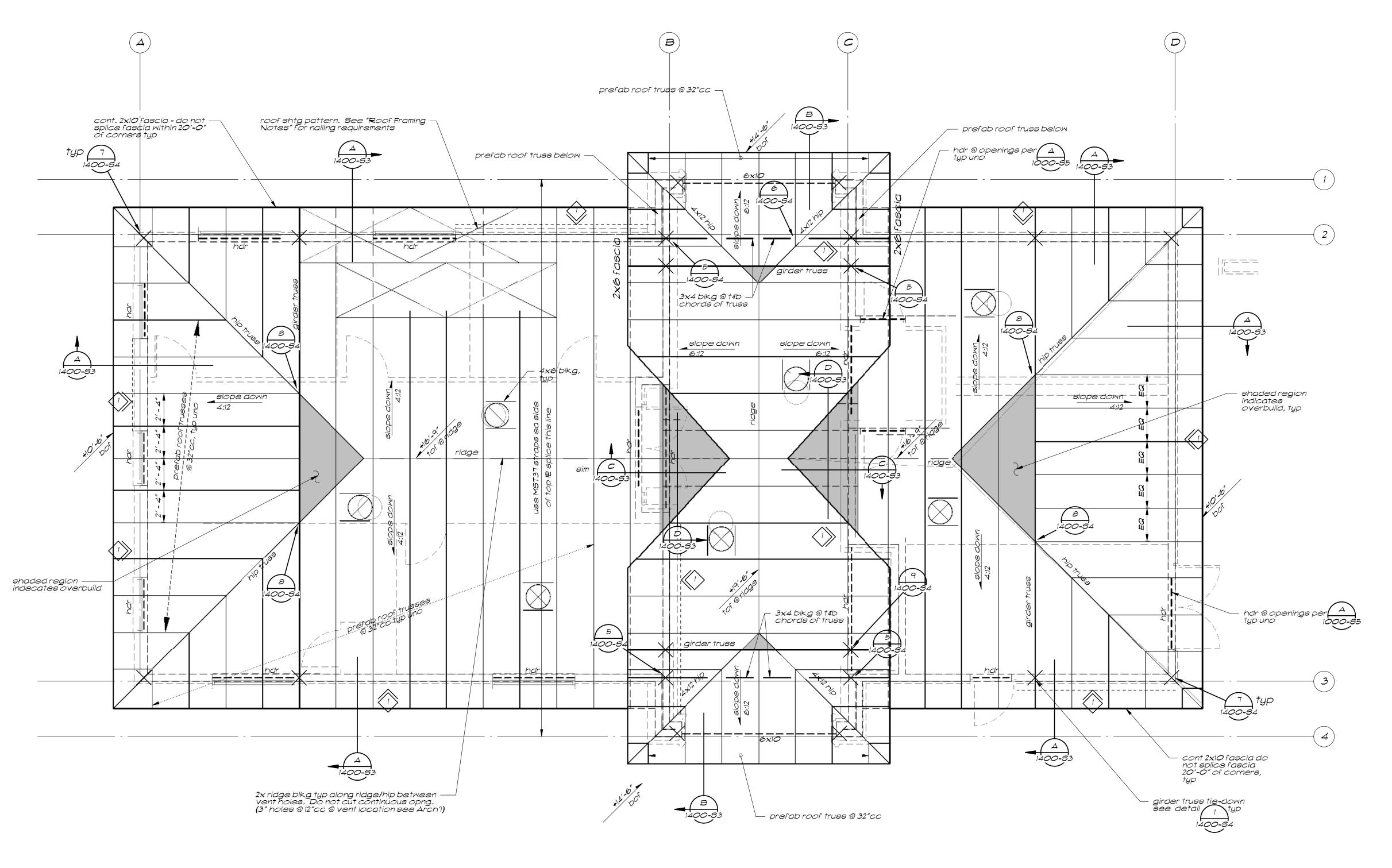
Subconsultant List	
DREYFUSS & BLACKFORD	3540 FOLSOM BOULEVARD, SACRAMENTO, CA 95816 (916) 453-1234
CAPITAL ENGINEERING CONSULTANTS	11020 SUN CENTER DRIVE, RANCHO CORDOVA, CA 95670 (916) 851-3500
ECOM ENGINEERING INC.	1796 TRIBUTE ROAD, SUITE 100 SACRAMENTO, CA (916) 641-5600
BUEHLER & BUEHLER STRUCTURAL ENGINEERS, INC.	600 Q STREET, STE. 200, SACRAMENTO, CA. 95811 (916) 443-0303
W.L. BURLE ENGINEERS, PA	111 S. WALNUT STREET, GREENVILLE, MS 38702 (662) 332-2619
AQUA ENGINEERING, INC.	4803 INNOVATION DRIVE, FORT COLLINS, CO 80525 (970) 229-9668
THE SIERRA WEST GROUP, LLC	2730 GATEWAY OAKS DRIVE, STE. 110 SACRAMENTO, CA. 95833 (916) 925-4000



P.I.C./ADMINISTRATION FOUNDATION PLAN	LOUISIANA GRAVESITE
Approved: Director, Office of Construction Management	Building Number

Approved: Director, Project Management Service

MARCH 22, 2012 A NATIONAL CEMETERY EXPANSION PHASE 1B 870CM3023B DRAWING NO. GAM 1400-S1 Dwg. 103 Of **137**





P.I.C./Administration Roof Framing Plan - 1/4" = 1'-0"

Roof Framing Plan Notes

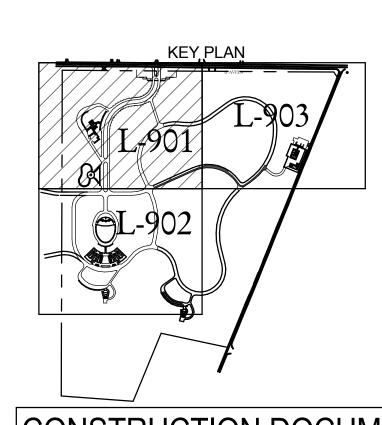
1. Roof sheathing shall be APA rated 3/4" shtg. Provide 10d common nails @ 6"cc at all supported edges and 10d @ 12" at all intermediate supports uno. Long dimension (Face grain of plywood shtg) shall be perpendicular to rafters typical uno. For roof diaphragm nlg, see 3. For typical roof opening framing, see 10

- 2. Verify all dimensions with Arch'l plans & elevations. Notify Architect immediately if there are any conflicts with the dimensions shown.
- Indicates post below, Size per foundation plan, Indicates HSS column below, Size per foundation plan,
- 4. ——— Indicates typical header per uno on plans or in sections.
- 5. For typical hdr to jamb stud support detail, see 3 uno.
- Indicates top of framing (t.o.f.) elevation (i.e., underside of shtg.), or t.o.w. (top of wall).
- 8. Verify sizes and locations of all openings with architectural drawings. It is the responsibility of the General Contractor to coordinate the exact size and location of all openings to be

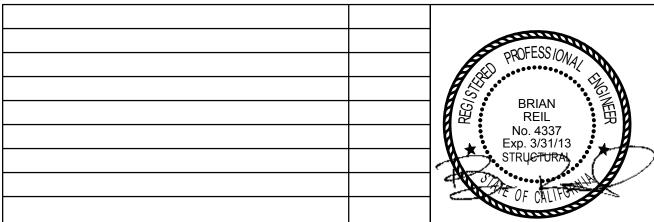
Indicates shear wall type below, See 🗔

- used by other trades. 9. The truss manufacturer/designer shall verify, in writing, that the uplift connectors shown in are acceptable for use with the prefabricated trusses.

 See sheet 1000-51 for Mood truss Notes.
- 10. For typical notes, details and schedules, see sheets 1000-51 to 1000-57.



CONSTRUCTION DOCUMENTS





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CAPITAL ENGINEERING CONSULTANTS	11020 SUN CENTER DRIVE, RANCHO CORDOVA, CA 95670 (916) 851-3500
ECOM ENGINEERING INC.	1796 TRIBUTE ROAD, SUITE 100 SACRAMENTO, CA (916) 641-5600
BUEHLER & BUEHLER STRUCTURAL ENGINEERS, INC.	600 Q STREET, STE. 200, SACRAMENTO, CA. 95811 (916) 443-0303
W.L. BURLE ENGINEERS, PA	111 S. WALNUT STREET, GREENVILLE, MS 38702 (662) 332-2619
AQUA ENGINEERING, INC.	4803 INNOVATION DRIVE, FORT COLLINS, CO 80525 (970) 229-9668
THE SIERRA WEST GROUP, LLC	2730 GATEWAY OAKS DRIVE, STE. 110 SACRAMENTO, CA. 95833 (916) 925-4000
i	



P.I.C./ADMINISTRATION ROOF FRAMING PLAN	LOUISIANA NATIONAL CEMETERY GRAVESITE EXPANSION PHASE 1B			
Approved: Director, Office of Construction Management	Building Number	Checked	Drawn	
		BOR	GAM	

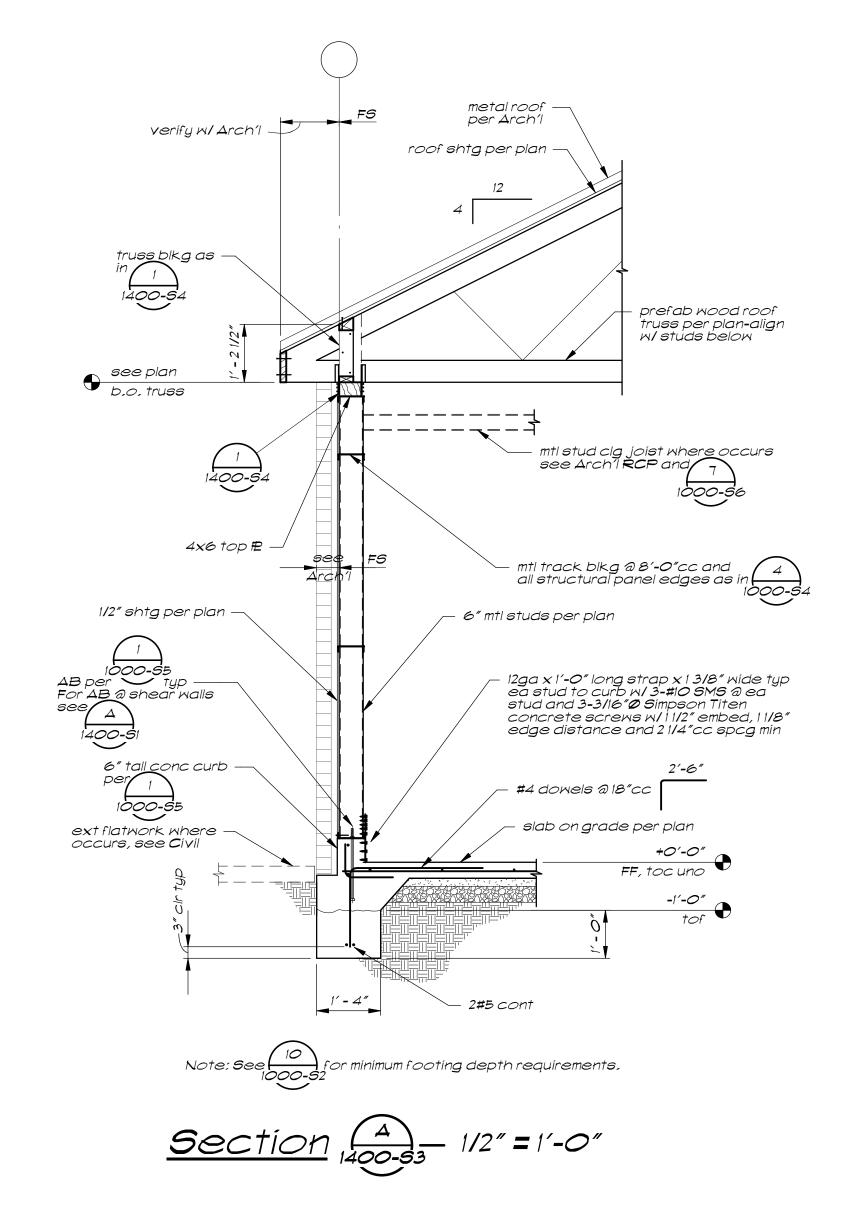
Approved: Director, Project Management Service

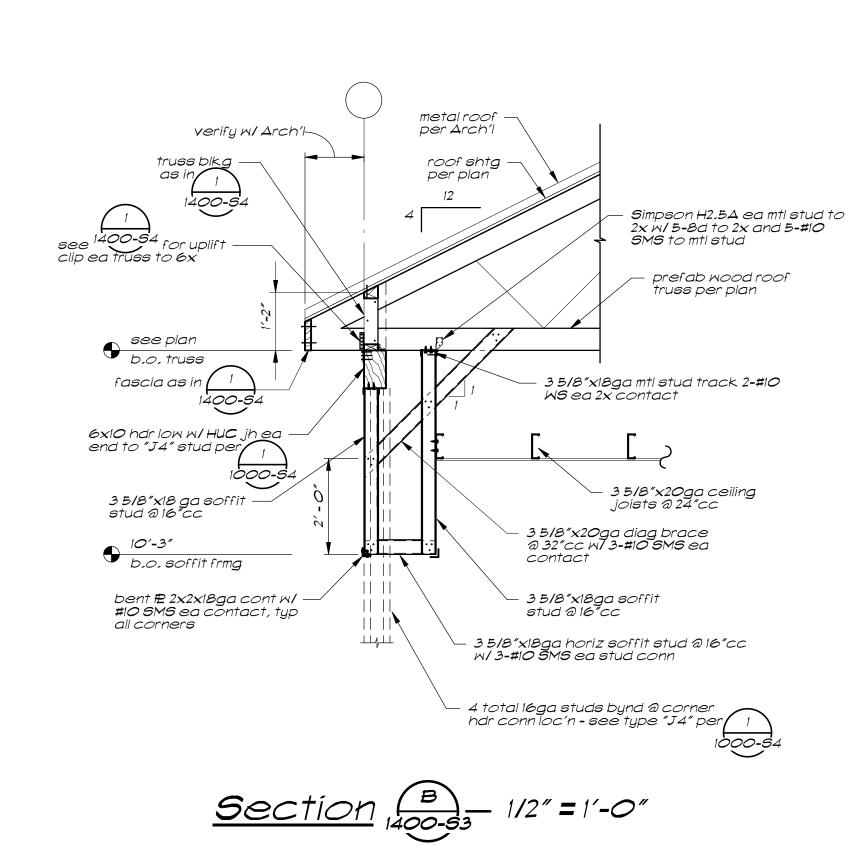
LOUISIANA NATIONAL CEMETERY ZACHARY, LOUISIANA

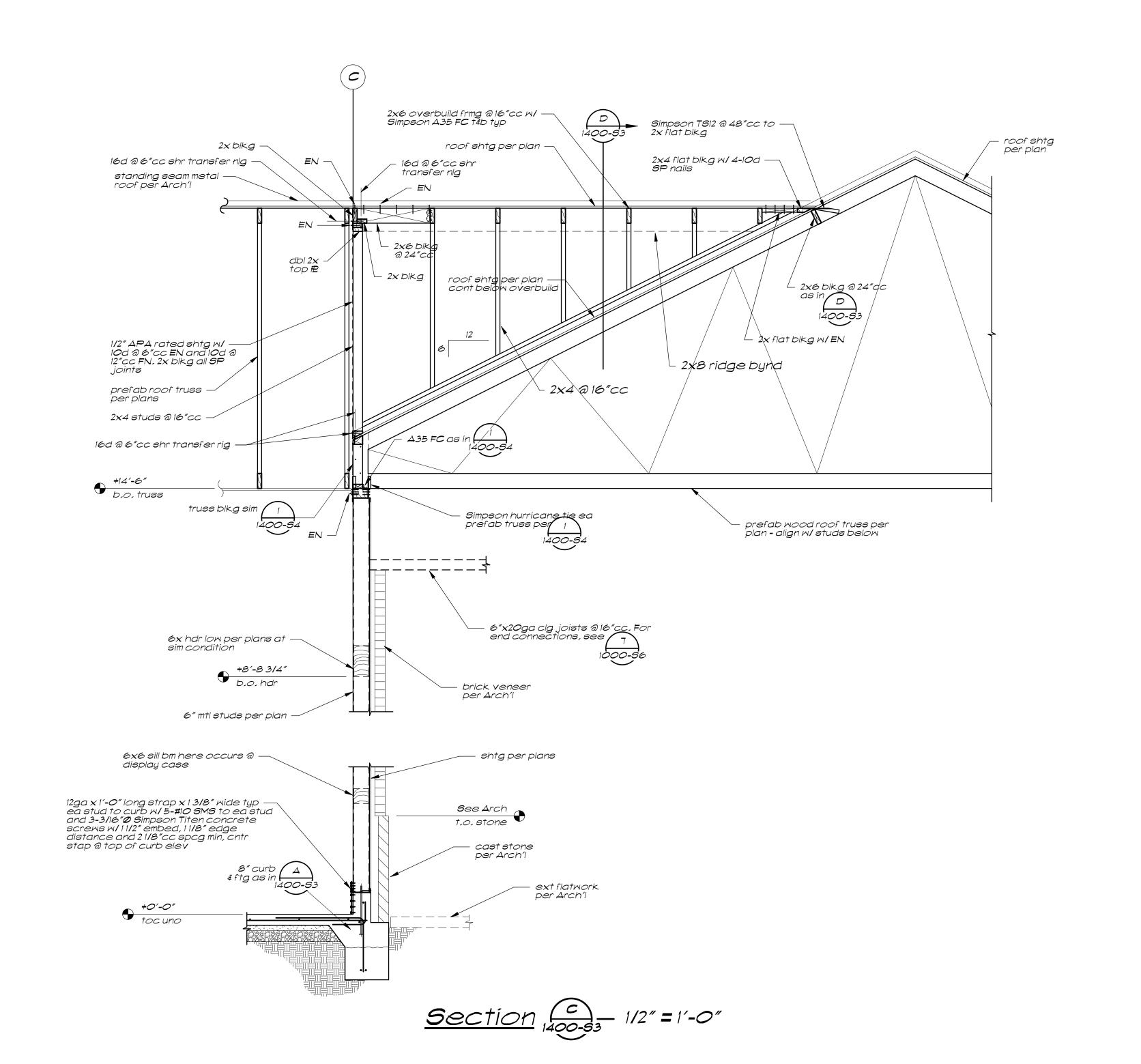
/ 1B	Project No. 870CM3023B
M	DRAWING NO. 1400-S2
	Dwg. 104 Of 137

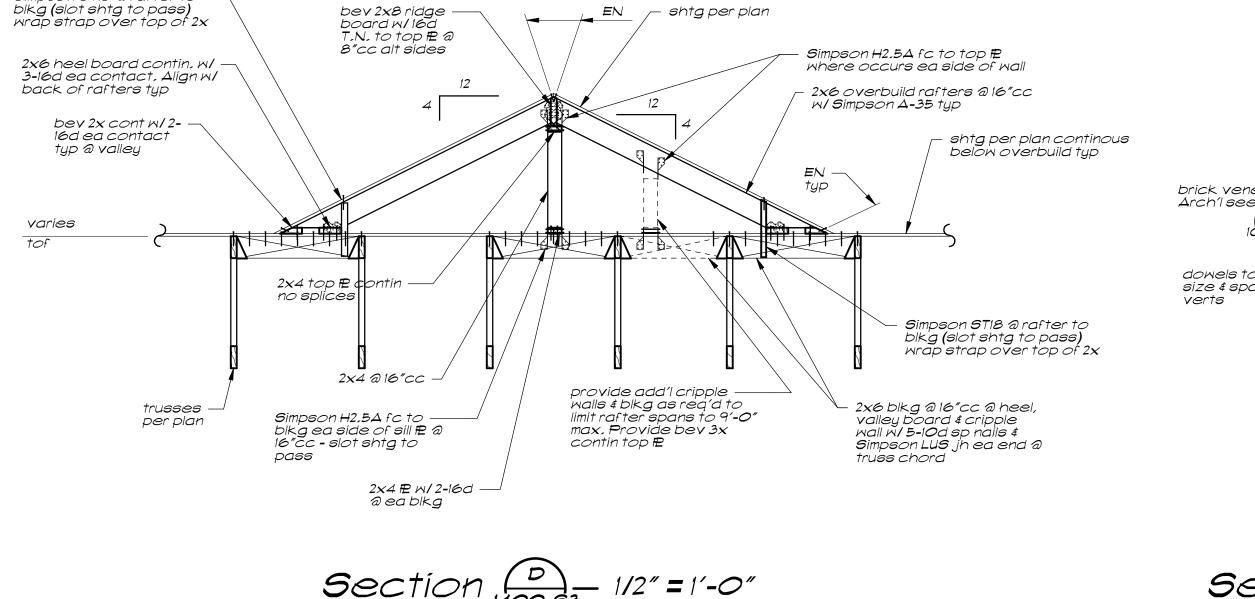
MARCH 22, 2012

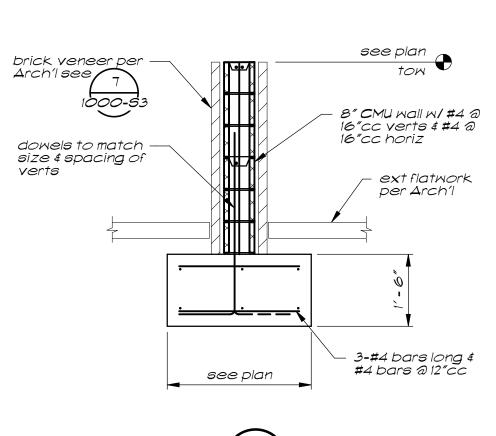




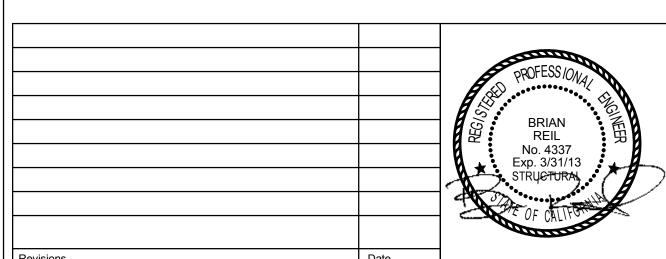








CONSTRUCTION DOCUMENTS



Simpson ST18 @ rafter to —

Structural Engineers, Inc. 600 Q Street, Suite 200, Sacramento, CA 9581 tel 916.443.0303 fax 916.443.0313

Carter Burgess
A Wholly Owned Subsidiary of Jacobs Consultants in Architecture, Engineering, Planning, and the Environment Sacramento Office 180 Promenade Circle, Suite 300

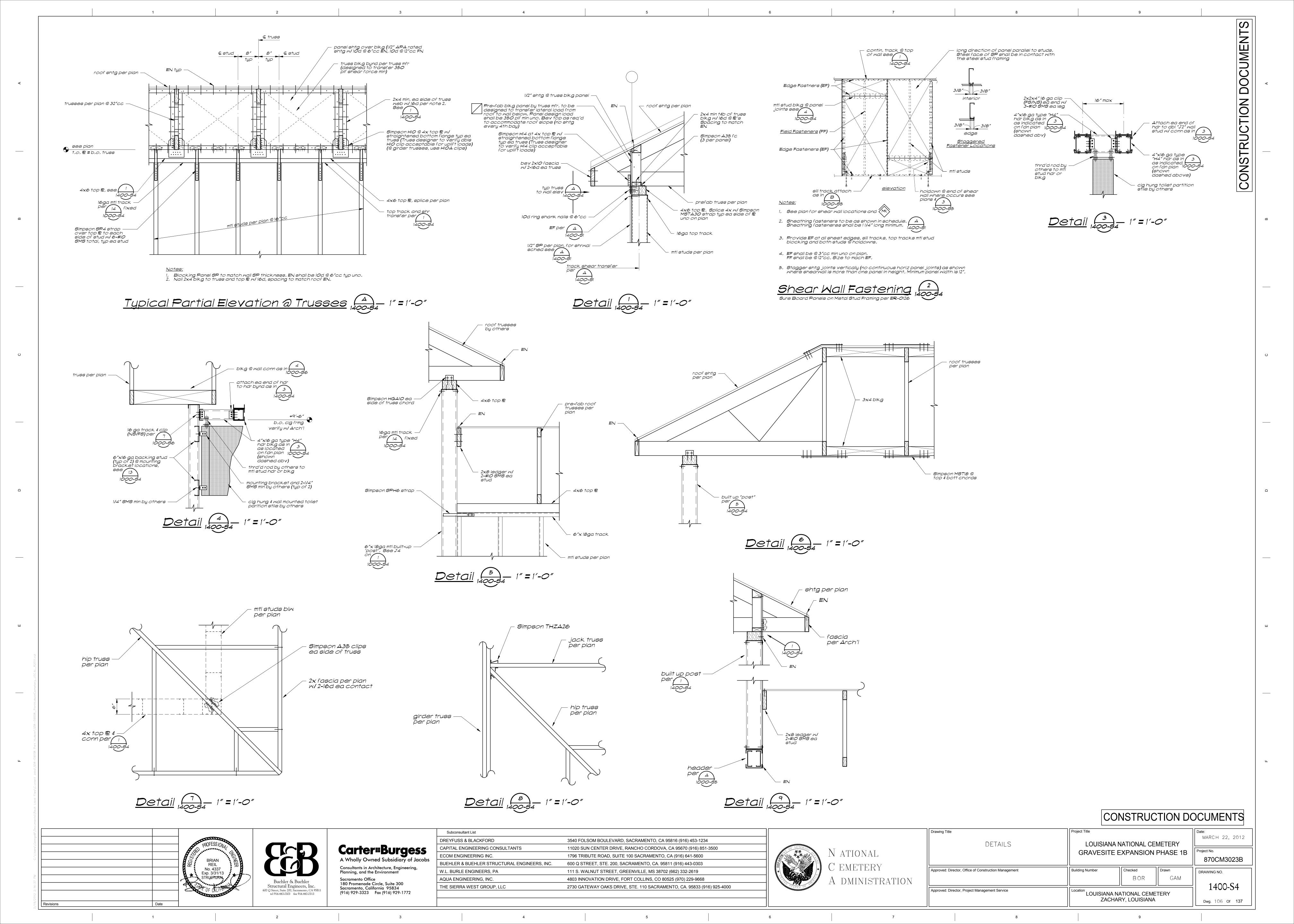
Sacramento, California 95834

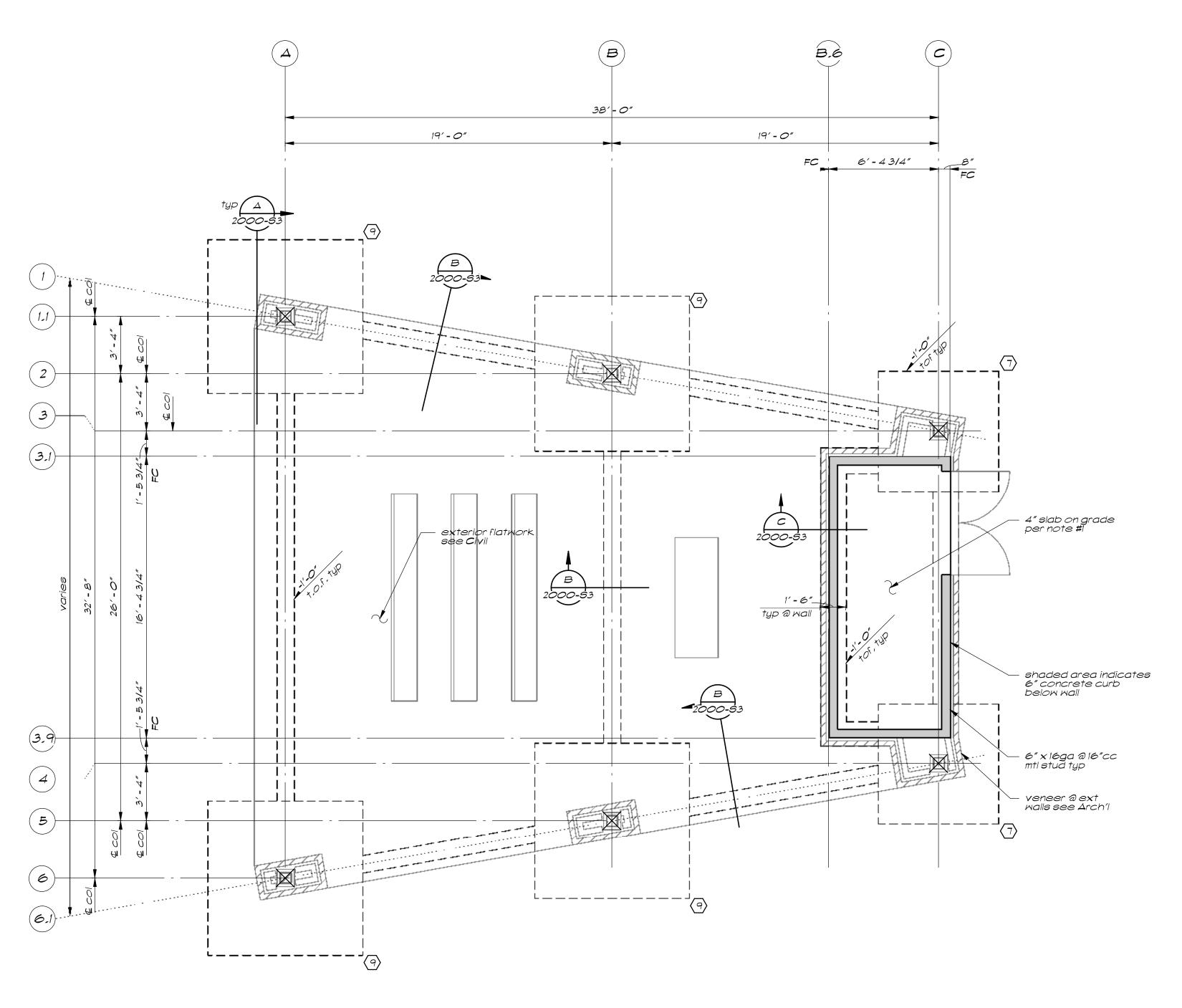
(916) 929-3323 Fax (916) 929-1772

Subconsultant List DREYFUSS & BLACKFORD 3540 FOLSOM BOULEVARD, SACRAMENTO, CA 95816 (916) 453-1234 CAPITAL ENGINEERING CONSULTANTS 11020 SUN CENTER DRIVE, RANCHO CORDOVA, CA 95670 (916) 851-3500 ECOM ENGINEERING INC. 1796 TRIBUTE ROAD, SUITE 100 SACRAMENTO, CA (916) 641-5600 BUEHLER & BUEHLER STRUCTURAL ENGINEERS, INC. 600 Q STREET, STE. 200, SACRAMENTO, CA. 95811 (916) 443-0303 W.L. BURLE ENGINEERS, PA 111 S. WALNUT STREET, GREENVILLE, MS 38702 (662) 332-2619 AQUA ENGINEERING, INC. 4803 INNOVATION DRIVE, FORT COLLINS, CO 80525 (970) 229-9668 2730 GATEWAY OAKS DRIVE, STE. 110 SACRAMENTO, CA. 95833 (916) 925-4000 THE SIERRA WEST GROUP, LLC



			MOIRUC		JCUIVIEIN I S
	Drawing Title SECTIONS	Project Title LOUISIANA	NATIONAL CE	METERY	Date: MARCH 22, 2012
		GRAVESITE	EXPANSION	PHASE 1B	Project No. 870CM3023B
	Approved: Director, Office of Construction Management	Building Number	Checked B O R	Drawn GAM	DRAWING NO. 1400-S3
Approved: Director, Project Management Service		LOUISIANA NATIONAL CEMETERY ZACHARY, LOUISIANA			1400-33 Dwg. 105 Of 137







Committal Shelter Foundation Plan - 1/4" = 1'-0"

Foundation Plan Notes

Typical slab on grade shall be 4" NW concrete w/ #4 @ 18"cc ea way @ mid-depth over 15 mil vapor barrier. See geotech report for pad preparation below vapor barrier. Provide slab on grade control joints as indicated in 1

- Reference top of concrete (toc) shall be +0'-0" typ uno. See Civil plans for datum elevation.
- Verify all dimensions with Architectural drawings. Notify
 Architect/Engineer of any discrepancies for resolution prior to
 proceeding. Dimensions are to face of stud (FS), uno. Face of
 concrete lines up w/ face of shtg.
- Mo Indicates pad footing per
- 5, Indicates HSS steel co HSS4x4x1/4 typ uno, Indicates H55 steel column. See plan for size.
- Indicates 6"x l6ga metal studs per plan, typ uno.
 All shear walls to have Sure-Board Series 200
 Structural Panels. See sheets 1000-54 & 1000-55
 for typical details. See

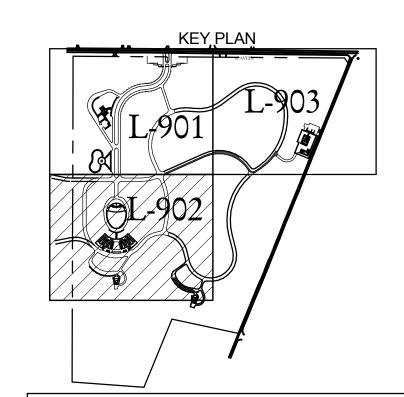


- Indicates elevation of top of footing below reference finished floor (toc) +0'-0", see plan.
- IO. Provide 3" minimum concrete cover at base £'s, AB's, and all other exposed steel below grade, typ.
- 11. For utility line (pipe & conduit) penetrations through footings 1000-52
- See Architectural and Civil drawings for grades, finishes and other requirements of all exterior flatwork, curbs, planters, etc.
- 13. See Architectural drawings for door and window details and locations, blockouts at door closers, curbs, depressions, pedestal dimensions, slopes and other like items.
- 14. Refer to sheets 1000-51 to 1000-57 for General Notes & Typical Details which are applicable to all drawings, uno.

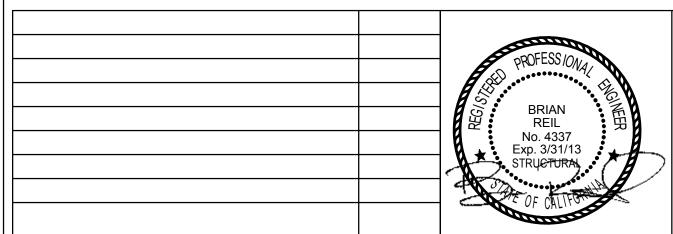
Footing Schedule						
				Metal Bldd	g Reactions	<u> </u>
<u>Mark</u>	<u>Síze</u>	<u>Reinforcing</u>	LC	Vert Down	<u>Vert Up</u>	<u>Thrust</u>
7	7'-0"\$ x 2'-6" thick	8-#8 EW (†#b)	D	-	-	-
			Lr	-	-	-
			W	-	-	_
			E	-	-	-
9	9'-0"\$ x 3'-0" thick	10-#8 EW († #b)	D	-	-	-
			Lr	-	-	_
			W	_	-	_

- 1. Rebar shall be 3" clear from bottom and sides of footing, 2" clear from top.
- Metal Building reactions listed are assumed, Loads to be verified prior to construction. See 'Typical Prefabricated Metal Building Notes' on sheet SI.O.
- 3. Unfactored reaction listed is at the interface of metal building column to conc slab or footing element.

4. Thrust indicates force acting in outward direction from building.



CONSTRUCTION DOCUMENTS





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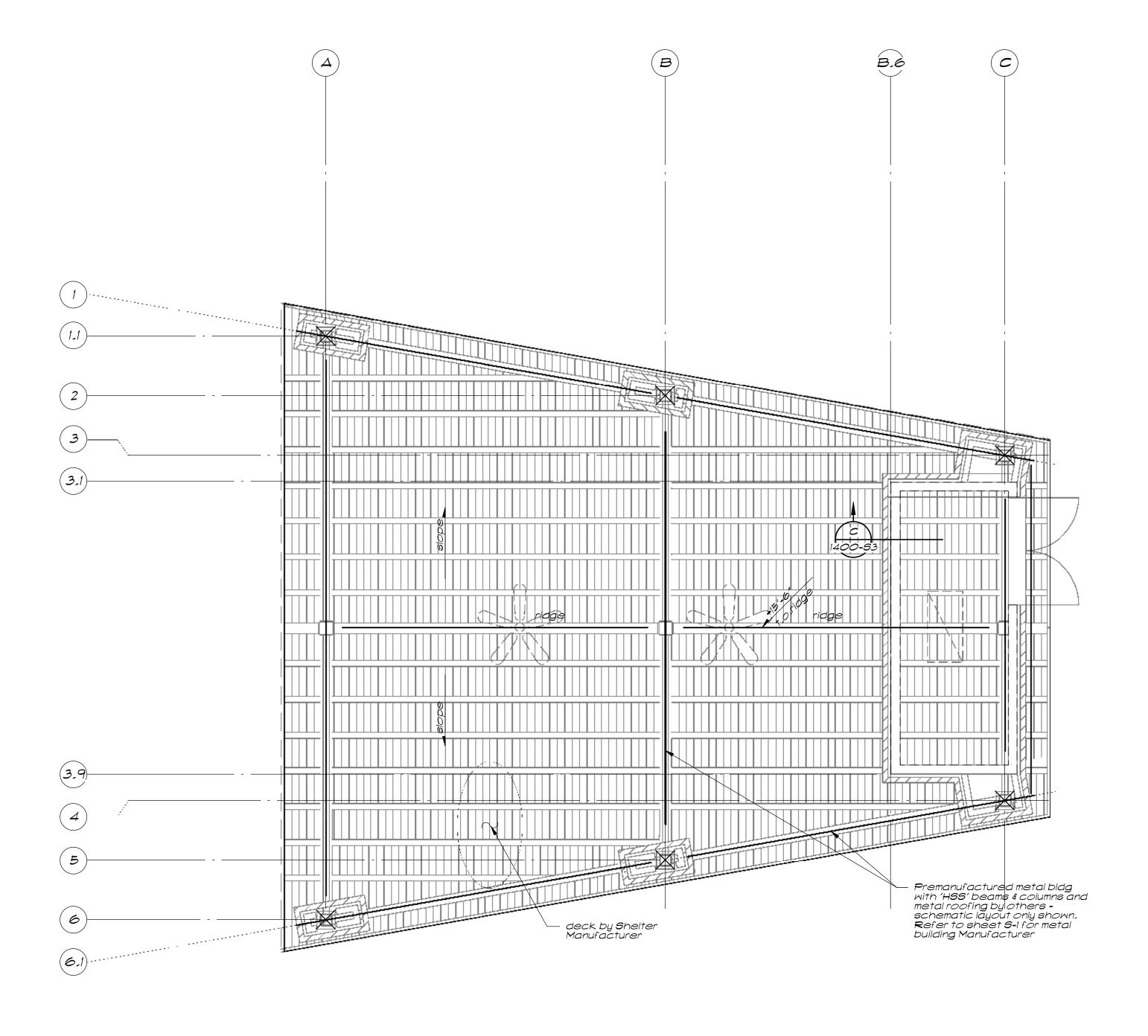
_		
Subconsultant List		
DREYFUSS & BLACKFORD	3540 FOLSOM BOULEVARD, SACRAMENTO, CA 95816 (916) 453-1234	
CAPITAL ENGINEERING CONSULTANTS	11020 SUN CENTER DRIVE, RANCHO CORDOVA, CA 95670 (916) 851-3500	
ECOM ENGINEERING INC.	1796 TRIBUTE ROAD, SUITE 100 SACRAMENTO, CA (916) 641-5600	
BUEHLER & BUEHLER STRUCTURAL ENGINEERS, INC.	600 Q STREET, STE. 200, SACRAMENTO, CA. 95811 (916) 443-0303	
W.L. BURLE ENGINEERS, PA	111 S. WALNUT STREET, GREENVILLE, MS 38702 (662) 332-2619	
AQUA ENGINEERING, INC.	4803 INNOVATION DRIVE, FORT COLLINS, CO 80525 (970) 229-9668	
THE SIERRA WEST GROUP, LLC	2730 GATEWAY OAKS DRIVE, STE. 110 SACRAMENTO, CA. 95833 (916) 925-4000	



	Drawing Title
	COMMITTAL SHELTER FOUN PLAN
Y	Approved: Director, Office of Construction Management
RATION	

E - - -

COMMITTAL SHELTER FOUNDATION PLAN		LOUISIANA NATIONAL CEMETERY GRAVESITE EXPANSION PHASE 1B			Date: MARCH 22, 2012
					Project No. 870CM3023B
	Approved: Director, Office of Construction Management	Building Number	Checked B O R	Drawn GAM	DRAWING NO.
Approved: Director, Project Management Service			ATIONAL CEME RY, LOUISIANA	TERY	2000-S1 Dwg. 107 Of 137



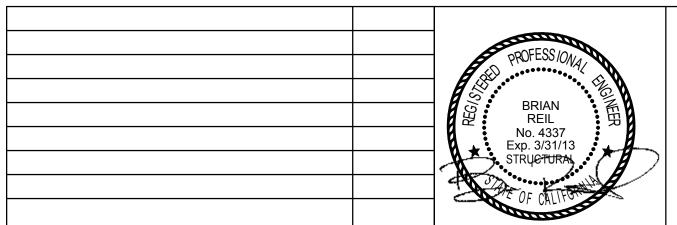


Committal Shelter Roof Framing Plan — 1/4" = 1'-0"

Roof Framing Plan Notes

- 1. For typical notes, details and schedules, see sheets 1000-51 to 1000-51,
- Verify all dimensions with Arch'l plans & elevations. Notify Architect immediately if there are any conflicts with the dimensions shown.
- 3, X Indicates steel column below per metal building mfr drawings,
- Arawings,
 4. Verify sizes and locations of all openings with architectural drawings. It is the responsibility of the General Contractor to coordinate the exact size and location of all openings to be used by other trades.

CONSTRUCTION DOCUMENTS





Carter Burges

A Wholly Owned Subsidiary of Jacobs

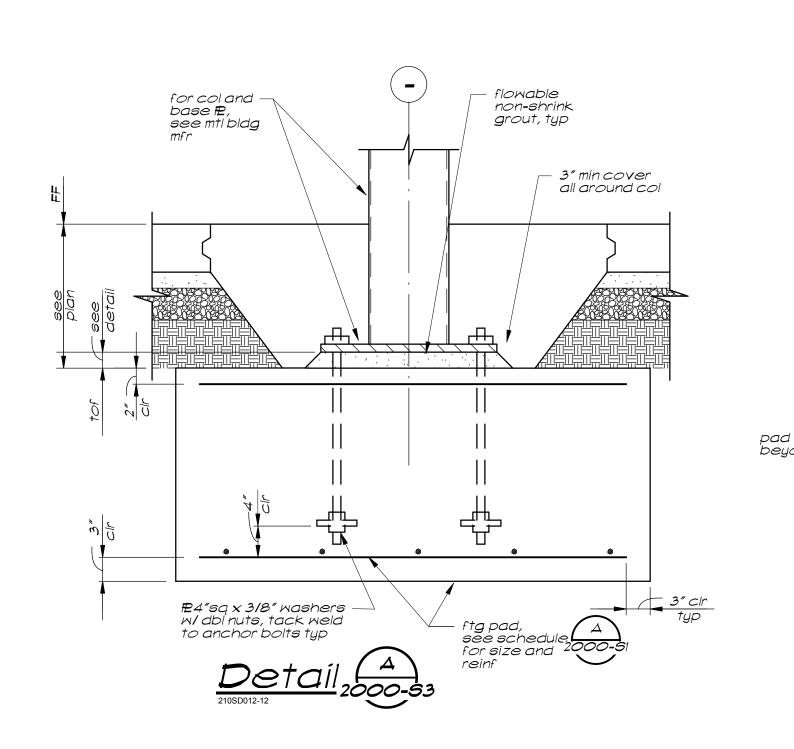
Consultants in Architecture, Engineering, Planning, and the Environment

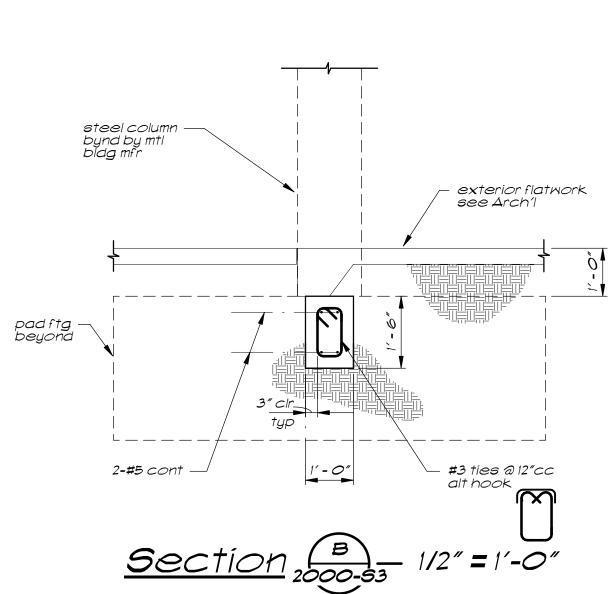
Sacramento Office
180 Promenade Circle, Suite 300
Sacramento, California 95834
(916) 929-3323 Fax (916) 929-1772

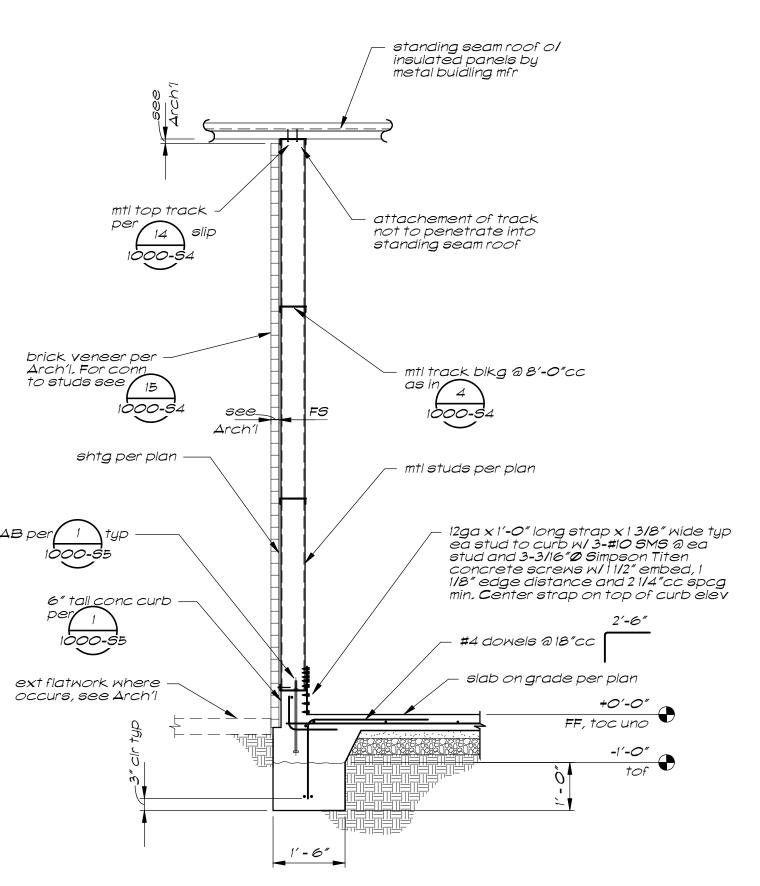
Subconsultant List	
DREYFUSS & BLACKFORD	3540 FOLSOM BOULEVARD, SACRAMENTO, CA 95816 (916) 453-1234
CAPITAL ENGINEERING CONSULTANTS	11020 SUN CENTER DRIVE, RANCHO CORDOVA, CA 95670 (916) 851-3500
ECOM ENGINEERING INC.	1796 TRIBUTE ROAD, SUITE 100 SACRAMENTO, CA (916) 641-5600
BUEHLER & BUEHLER STRUCTURAL ENGINEERS, INC.	600 Q STREET, STE. 200, SACRAMENTO, CA. 95811 (916) 443-0303
W.L. BURLE ENGINEERS, PA	111 S. WALNUT STREET, GREENVILLE, MS 38702 (662) 332-2619
AQUA ENGINEERING, INC.	4803 INNOVATION DRIVE, FORT COLLINS, CO 80525 (970) 229-9668
THE SIERRA WEST GROUP, LLC	2730 GATEWAY OAKS DRIVE, STE. 110 SACRAMENTO, CA. 95833 (916) 925-4000

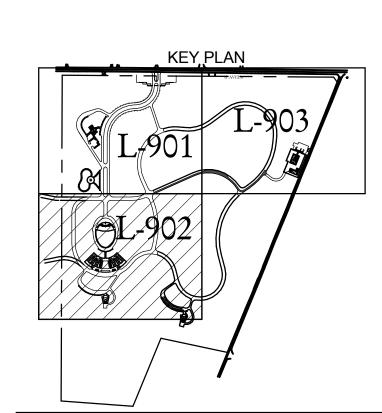


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Drawing Title COMMITTAL SHELTER ROOF FRAMING	Project Title LOUISIANA	NATIONAL CE	METERY	Date: MARCH 22, 2012
PLAN	GRAVESITE			Project No. 870CM3023B
Approved: Director, Office of Construction Management	Building Number	Checked B O R	Drawn GAM	DRAWING NO.
Approved: Director, Project Management Service	11	IATIONAL CEME RY, LOUISIANA		2000-S2 Dwg. 108 Of 137
	ZACHA	RY, LOUISIANA		Dwg. 108 Of 137

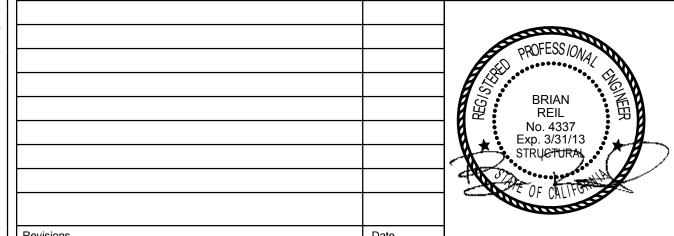








CONSTRUCTION DOCUMENTS



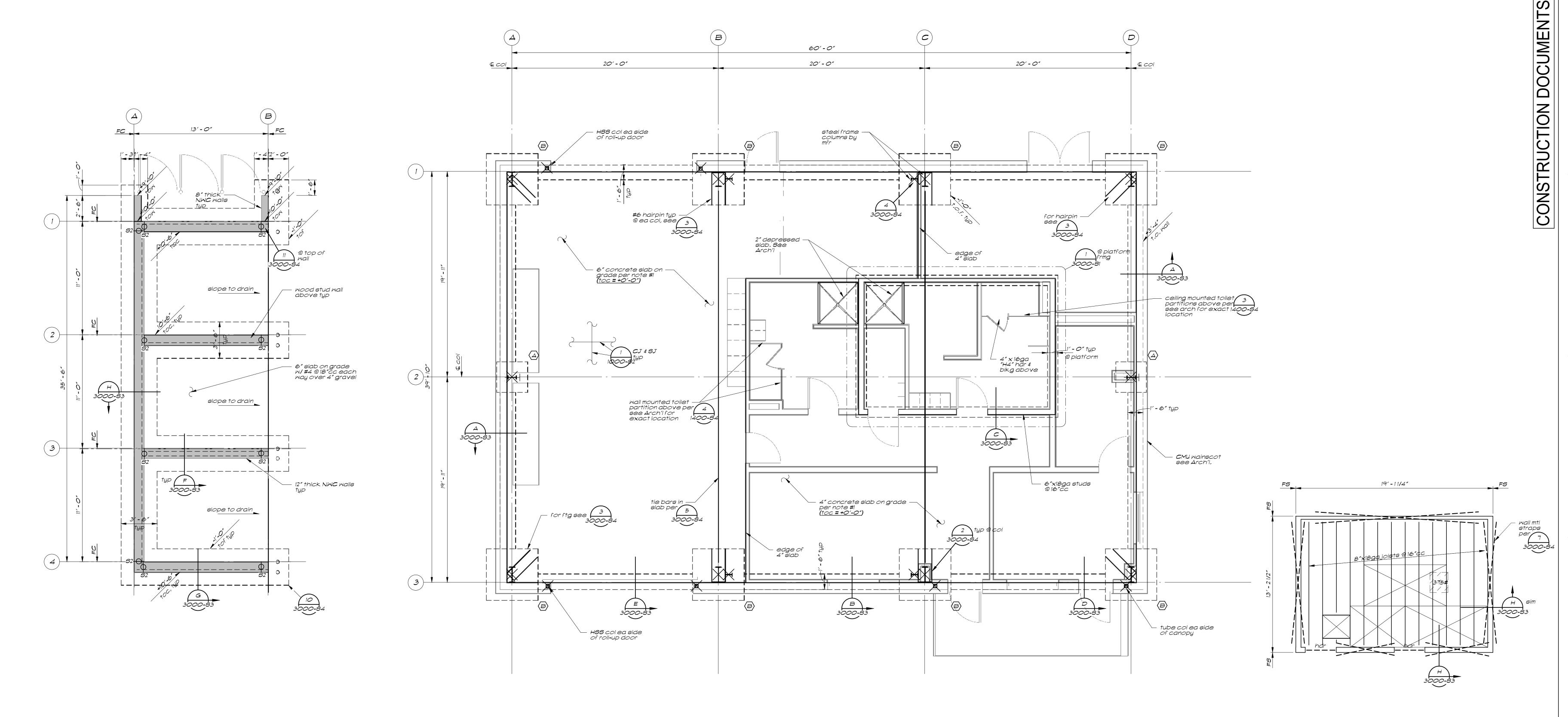


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SECTIONS/DETAILS	Project Title LOUISIANA NATIONAL CEMETERY		Date: MARCH 22, 2012	
,	GRAVESITE EXPANSION PHASE 1B			Project No.
				870CM3023B
oved: Director, Office of Construction Management	Building Number	Checked	Drawn	DRAWING NO.
		BOR	GAM	
oved: Director, Project Management Service	LOUISIANA NATIONAL CEMETERY			2000-S3
	ZACHARY, LOUISIANA		Dwg. 109 Of 137	





Bulk Storage Foundation Plan - 1/4" = 1'-0" BUILDING 3001 ADD-ALTERNATE #2

Footing Schedule						
				<u>Metal Blac</u>	g Reactions	<u>2</u>
<u>Mark</u>	<u>Síze</u>	Reinforcing	LC	Vert Down	<u>Vert Up</u>	<u>Thrust</u>
$\langle \Delta \rangle$	4'-0"\$\tau x 1'-6" thick	4-#5 EW (b)	D	-	-	-
		, ,	Lr	-	-	-
			W	-	-	-
			E	-	-	-
$\langle \mathcal{B} angle$	5'-0"# x 1'-6" thick	5-#5 EW (†#b)	D	-	-	-
			Lr	-	-	-
			W	-	-	-
1			=			

- <u>dvNotes:</u> 1. Rebar shall be 3" clear from bottom and sides of footing, 2" clear from top.
- Metal Building reactions listed are assumed, Loads to be verified prior to construction. See 'Typical Prefabricated Metal Building Notes' on sheet 1000-51.
- 3. Unfactored reaction listed is at the interface of metal building column to conc slab or

4. Thrust indicates force acting in outward direction from building.



<u>Foundation Plan Notes</u>

typical for all interior slabs. Construction joints may replace control joints as required.

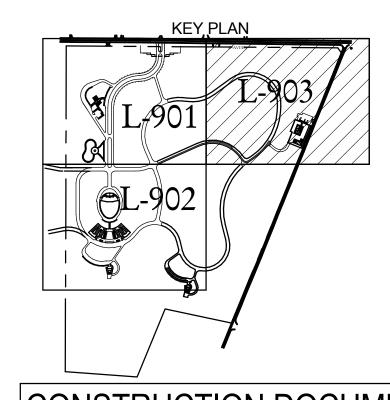
- Reference top of concrete (toc) shall be +0'-0" typ uno. See Civil plans for datum elevation. 3. Verify all dimensions with Architectural drawings. Notify Architect/Engineer of any discrepancies for resolution prior to proceeding. Dimensions are to face of stud (FS), uno. Face of
- concrete lines up w/face of shtg. 4. (Ms) Indicates pad footing per $\begin{pmatrix} A \\ 3000-51 \end{pmatrix}$ Footing to be centered on main WF col, uno.
- Indicates HSS column, See metal building mfr. Indicates WF column, See metal building mfr.
- 6. Indicates léga built-up metal stud For type see $\begin{pmatrix} 1 \\ J2 \end{pmatrix}$ See plan for type callout and loc'n. 1000-54
- 7. Indicates 2x6 wood studs per plan, typ uno. All walls to have 1/2"shtg. See sheet 1000-57 for typical details,

- 8. Indicates 8" CMU wall. For reinf & typical details see sheet 1000-53
- 17. Φ Indicates Simpson HDU type holdowns, see Θ (i.e. H5 = HDU5).
- Indicates elevation of top of footing below reference finished floor (toc) +0'-0", see plan. 11. Provide 3" minimum concrete cover at base 12's, AB's, and all
- other exposed steel below grade typ. 12. For utility line (pipe & conduit) penetrations through footings
- See Architectural and Civil drawings for grades, finishes and other requirements of all exterior flatwork, curbs, planters,
- 14. See Architectural drawings for door and window details and locations, blockouts at door closers, curbs, depressions, pedestal dimensions, slopes and other like items. 15. Refer to sheets 1000-51 thru 1000-57 for General Notes & Typical Details which are applicable to all drawings, uno.

conditions, MBM to coordinate dims accordingly,

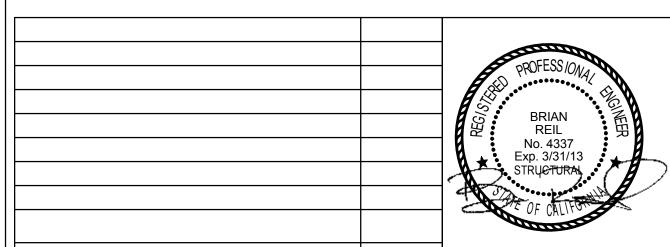
16. <u>hdr</u> Indicates typ hdr per D





CONSTRUCTION DOCUMENTS

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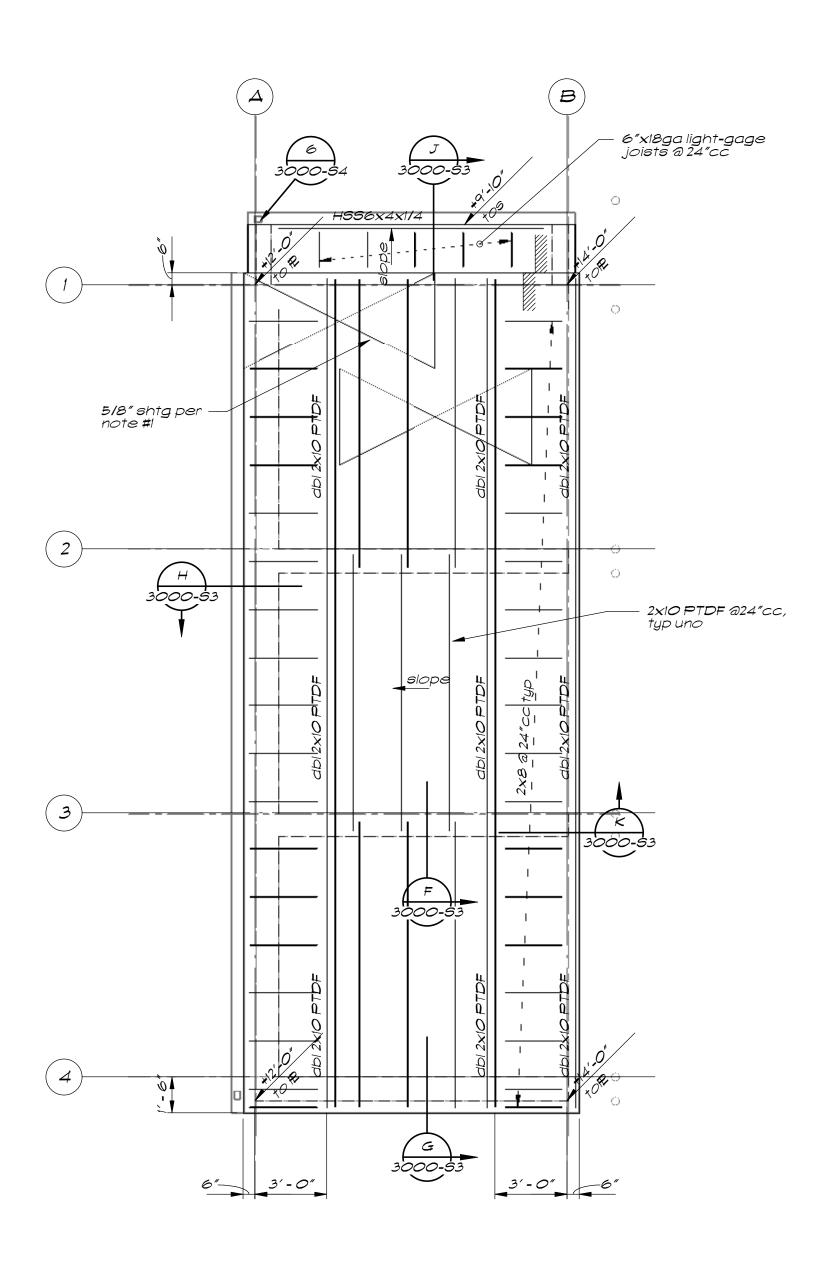
	Subconsultant List		
	DREYFUSS & BLACKFORD	3540 FOLSOM BOULEVARD, SACRAMENTO, CA 95816 (916) 453-1234	
	CAPITAL ENGINEERING CONSULTANTS	11020 SUN CENTER DRIVE, RANCHO CORDOVA, CA 95670 (916) 851-3500	
	ECOM ENGINEERING INC.	1796 TRIBUTE ROAD, SUITE 100 SACRAMENTO, CA (916) 641-5600	
S	BUEHLER & BUEHLER STRUCTURAL ENGINEERS, INC.	600 Q STREET, STE. 200, SACRAMENTO, CA. 95811 (916) 443-0303	
	W.L. BURLE ENGINEERS, PA	111 S. WALNUT STREET, GREENVILLE, MS 38702 (662) 332-2619	
	AQUA ENGINEERING, INC.	4803 INNOVATION DRIVE, FORT COLLINS, CO 80525 (970) 229-9668	
	THE SIERRA WEST GROUP, LLC	2730 GATEWAY OAKS DRIVE, STE. 110 SACRAMENTO, CA. 95833 (916) 925-4000	



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Drawing Title	Project Title			Date:
MAINTENANCE & BULK STORAGE	 LOUISIANA I	NATIONAL CE	EMETERY	MARCH 22, 2012
FOUNDATION PLAN	GRAVESITE E	EXPANSION	PHASE 1B	Project No.
				870CM3023B
Approved: Director, Office of Construction Management	Building Number	Checked	Drawn	DRAWING NO.
		BOR	GAM	
Approved: Director, Project Management Service	LOUISIANA N.	ATIONAL CEM	ETERY	3000-S1

ZACHARY, LOUISIANA

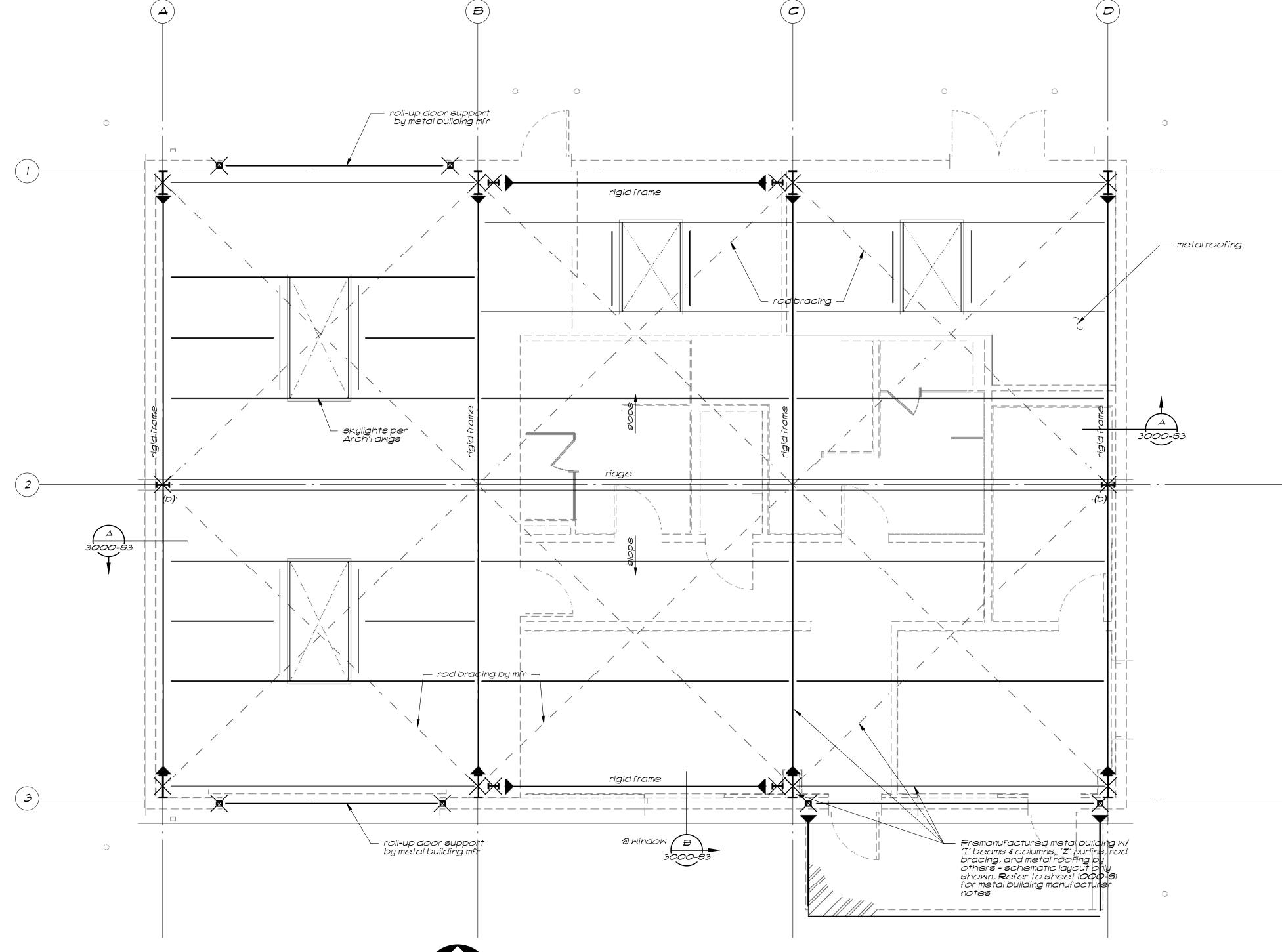




Bulk Storage Roof Framing Plan — 1/4" = 1'-0"
BUILDING 3001
ADD-ALTERNATE #2

Roof Framing Plan Notes

- 1. Roof sheathing shall be APA rated 5/8" shtg. Provide 10d common nails @ 6"cc at all supported edges and 10d @ 12" at all intermediate supports uno. Long dimension (Face grain of plywood shtg) shall be perpendicular to rafters typical uno. For roof diaphragm nlg, see 3 For typical roof opening framing, see
- 2. Verify all dimensions with Arch'l plans & elevations, Notify Architect immediately if there are any conflicts with the
- Indicates post below, Size per foundation plan, Indicates HSS column below, Size per foundation plan,
- 4. ——— Indicates typical header per uno on plans or in sections.
- 5. For typical hdr to jamb stud support detail, see $\begin{pmatrix} 3 \\ 1000-54 \end{pmatrix}$ uno.
- Indicates top of framing (t.o.f.) elevation (i.e., underside of shtg.), or t.o.w. (top of wall). Indicates shear wall type below. See A
- 8. Verify sizes and locations of all openings with architectural drawings. It is the responsibility of the General Contractor to coordinate the taxact size and location of all openings to be
- 9. For typical notes, details and schedules, see sheets 1000-51 to 1000-1007



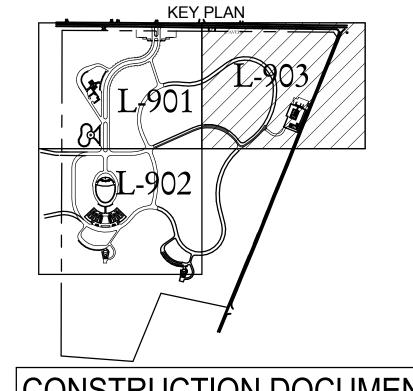


Maint Roof Framing Plan — 1/4" = 1'-0"
BUILDING 3000
ADD-ALTERNATE #1

Roof Framing Plan Notes

used by other trades.

- Verify all dimensions with Arch'l plans & elevations. Notify Architect immediately if there are any conflicts with the dimensions shown.
- Indicates **KF** col below per metal building mfr drawings,
- 3. Verify sizes and locations of all openings with architectural drawings. It is the responsibility of the General Contractor to coordinate the exact size and location of all openings to be



CONSTRUCTION DOCUMENTS

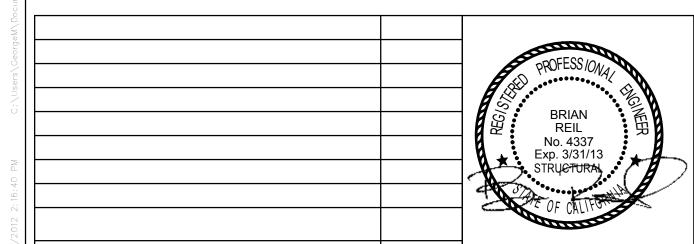
MARCH 22, 2012

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DRAWING NO.





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awing Title	Project Title
MAINTENANCE & BULK STORAGE ROOF FRAMING PLAN	LOUISIANA NATIONAL CEMETERY GRAVESITE EXPANSION PHASE 1B

Approved: Director, Office of Construction Management Building Number BOR GAM LOUISIANA NATIONAL CEMETERY ZACHARY, LOUISIANA Approved: Director, Project Management Service

